

September 7, 2016

Geotechnical Soils Investigation Report

**Parking Deck
200 K Street, NW
Washington, DC**



**GeoCapitol
Engineering LLC**

4545 42nd Street, NW
Suite 307
Washington, DC 20016
(202) 375-7900 • www.geocapeng.com



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Suite 307
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(202) 375-7900
www.geocapeng.com

September 7, 2016

Mr. Mark Corneal
Office of the Deputy Mayor for Planning and Economic Development (DMPED)
1350 Pennsylvania Ave, NW, Suite 317
Washington, DC 20004

**Subject: Geotechnical Soils Investigation Report, Parking Deck,
200 K Street, NW, Washington, DC (GeoCapitol Project
No. DC16024)**

Dear Mr. Corneal:

GeoCapitol Engineering LLC (GeoCapitol) is pleased to present the following geotechnical soils investigation report prepared for Parking Deck, at 200 K Street, NW, Washington, DC.

We appreciate the opportunity to serve as your geotechnical consultant on this project. Please do not hesitate to contact me if you have any questions or want to meet to discuss the findings and recommendations contained in the report.

Sincerely,

GEOCAPITOL ENGINEERING LLC

A handwritten signature in blue ink, appearing to read "Daniel F. Gradishar". The signature is fluid and cursive, with a long horizontal stroke at the end.

Daniel F. Gradishar, PE
President
dgradishar@geocapeng.com

Table of Contents

1.0 Scope of Services	1
2.0 Site Description and Proposed Construction	1
3.0 Subsurface Conditions	1
3.1 Geology.....	2
3.2 Published Soils.....	2
3.3 Stratification	2
3.4 Groundwater	3
3.5 Soil Laboratory Test Results.....	3
3.6 Seismic Site Classification	4
4.0 Engineering Analysis.....	4
4.1 Rock Excavation.....	4
4.2 Adding Loads to the Existing Foundations.....	4
5.0 General Limitations.....	4

Figure 1: Site Vicinity Map

Appendix A: Subsurface Investigation

Appendix B: Soil Laboratory Test Results

1.0 Scope of Services

This geotechnical soils investigation report presents the results of the field investigation, soil laboratory testing, and engineering analysis of the geotechnical data. This report specifically addresses the following:

- An evaluation of subsurface conditions within the area of the proposed site development, including a seismic site classification and site-specific spectral response.
- Comments on subsurface rock near the structure footprint.
- Comments on utilizing existing foundations for support of the proposed structure addition.

Services not specifically identified in the contract for this project are not included in the scope of services.

2.0 Site Description and Proposed Construction

The site is located at 200 K Street, NW, Washington, DC and consists of an existing parking deck straddling the I-395 tunnel. A site vicinity map is presented as Figure 1 at the end of this report. The ground surface elevation at the site ranges from EL 44 to 50 feet. Based on RFQ dated April 22, 2016, the proposed construction consists of an addition to the top parking deck on the existing parking garage structure. We understand from the review of the existing as-built drawings provided by DDOT and Carmel Plaza Apartments that the parking garage structure was originally designed to have townhouses or other residential structures located where the existing parking deck is being utilized to park vehicles off of 2nd Street, NW.



Note: Aerial Photograph from Google Earth Imagery Dated 2016.

3.0 Subsurface Conditions

Subsurface conditions were investigated by drilling a total of two (2) test borings in the proposed site development area. Test boring logs and a boring location plan are presented in Appendix A of this report.

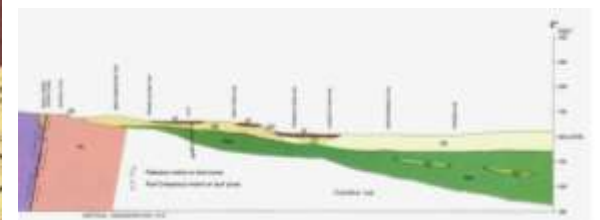
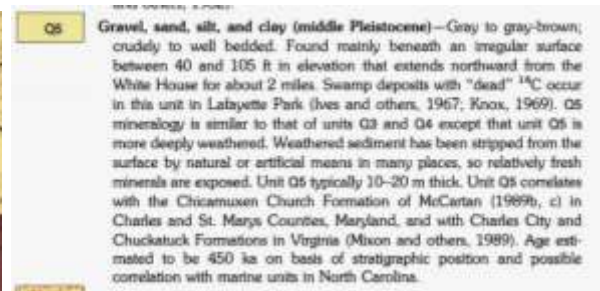
Previous geotechnical engineering soil test borings completed during the design of the I-395 tunnel were provided by DDOT and reviewed by us. The soil test borings were completed by Tippetts-Abbott-McCarthy-Stratton Engineers and Architects (TAMSEA) dated April 1974. Although a geotechnical engineering report was not discovered, the subsurface information from that study has been reviewed as a part of this project.

Pertinent previous test borings by TAMSEA are also included in Appendix A of this report. Test boring data by others is assumed to be complete and accurate. We do not assume any responsibility for the completeness and accuracy of data obtained from others without our supervision.

3.1 Geology

The site is located within the Coastal Plain Physiographic Province of the District of Columbia. The Coastal Plain consists of a seaward thickening wedge of unconsolidated to semi-consolidated sedimentary deposits from the Cretaceous Geologic Period to the Holocene Geologic Epoch. These deposits represent marginal-marine to marine sediments consisting of interbedded sands and clays. The Coastal Plain is bordered to the east by the Atlantic Ocean and to the west by the Piedmont Physiographic Province. The dividing line between the Coastal Plain and the Piedmont is locally referred to as the “Fall Line”. This name comes from the waterfalls that form as a result of the differential erosion that occurs as streams cross the Piedmont/Coastal Plain contact.

Specifically, according to local geologic maps, the site is mapped in the Q5 gravel, sand, silt, and clay deposits of the Quaternary geologic period underlain by the clay-dominated lithofacies of the Potomac Formation of the Cretaceous geologic period. The Potomac Group sediments are the oldest sedimentary deposits in the Washington, DC area. These soils are known to be highly over-consolidated as a result of the weight of a substantial thickness of overlying soils that have since been eroded away. As a result of over-consolidation, Potomac Group soils have been pre-loaded and are capable of supporting substantial loads. The Potomac Group clays are well documented with problems associated with slope instability and excessive shrink/swell characteristics.



Map excerpt provided courtesy of the USGS (Fleming, A.H., Drake, A.A., and McCartan, Lucy, 1994, Geologic map of the Washington West quadrangle, District of Columbia, Montgomery and Prince George’s Counties, Maryland, and Arlington and Fairfax Counties, Virginia: U.S. Geological Survey)

3.2 Published Soils

A review of the DC soils map on the DC Atlas Plus website and the NRCS-USDA Web Soil Survey indicates the site is mapped within the urban land soils.

3.3 Stratification

The subsurface materials encountered have been stratified for purposes of our discussions herein. These stratum designations do not imply that the materials encountered are continuous across the site. Stratum designations have been established to characterize similar subsurface conditions based on material

gradations and parent geology. The generalized subsurface materials encountered in the test borings completed at the site have been assigned to the following strata:

Stratum A (Existing Fill)	generally soft to firm or loose, poorly graded gravel, well graded sand, and clayey sand, FILL, with various amounts of concrete, organics, and sand, dry to wet, brown, yellow, and gray
Stratum B1 (Potomac Group)	firm to stiff, LEAN CLAY (CL), with sand and micaceous, wet, gray
Stratum B2 (Potomac Group)	Loose to hard, clayey SAND (SM) and POORLY GRADED SAND (SM), moist, yellow, red, and gray

The two letter designations included in the strata descriptions presented above and on the test boring logs represent the Unified Soil Classification System (USCS) group symbol and group name for the samples based on laboratory testing per ASTM D-2487 and visual classifications per ASTM D-2488. It should be noted that visual classifications per ASTM D-2488 may not match classifications determined by laboratory testing per ASTM D-2487.

3.4 Groundwater

Groundwater level observations were made in the field during drilling. Groundwater was encountered at a depth of about 40 feet below the existing ground surface, or EL 10 at B-2.

The groundwater observation presented herein is considered to be an indication of the groundwater level at the date and time indicated. Where more impervious clay soils are encountered, the amount of water seepage into the borings is limited, and it is generally not possible to establish the location of the groundwater table through short term water level observations. Accordingly, the groundwater information presented herein should be used with caution. Also, fluctuations in groundwater levels should be expected with seasons of the year, construction activity, and changes to surface grades, precipitation, or other similar factors.

3.5 Soil Laboratory Test Results

Selected soil samples obtained from the field investigation were tested for grain size distribution, Atterberg limits, and natural moisture contents. A summary of soil laboratory test results is presented below, and the results of natural moisture content tests are presented on the test boring logs in Appendix A.

Test Boring No.	Depth (ft)	Sample Type	Stratum	Description of Soil Specimen	Sieve Results		Atterberg Limits			Natural Moisture Content (%)
					Percent Retained #4 Sieve	Percent Passing #200 Sieve	LL	PL	PI	
B-1	23.5-25	Split-spoon sample	A	WELL GRADED SAND with silt and gravel (SW-SM)	19.3	8.7	NP	NP	NP	6.8
B-2	5-6.5	Split-spoon sample	A	CLAYEY SAND (SC)	0.2	23.6	51	16	35	13.1
B-2	23.5-25	Split-spoon sample	B1	LEAN CLAY with sand (CL)	0	82.1	40	16	24	22.1

Test Boring No.	Depth (ft)	Sample Type	Stratum	Description of Soil Specimen	Sieve Results		Atterberg Limits			Natural Moisture Content (%)
					Percent Retained #4 Sieve	Percent Passing #200 Sieve	LL	PL	PI	
B-2	53.5-55	Split-spoon sample	B2	CLAYEY SAND (SC)	0	21.9	36	20	16	26.9

Notes:

1. Soil tests are in accordance with applicable ASTM standards
2. Soil classification symbols are in accordance with Unified Soil Classification System
3. Visual identification of samples is in accordance with ASTM D-2488
4. Key to abbreviations: LL = liquid limit; PL = plastic limit; PI = plasticity index; NP = non-plastic; N/T = not tested

3.6 Seismic Site Classification

Based on the results of the subsurface investigation and our knowledge of local geologic conditions, the site soils have been assigned to a site class D per the International Building Code (IBC). It may be possible to improve the site classification from a "D" to a "C" with in-situ shear wave testing at the site. We can provide this additional service upon request.

4.0 Engineering Analysis

Recommendations regarding utilizing the existing foundations for the proposed added load are presented herein.

4.1 Rock Excavation

Rock was not encountered in the recent borings to depths of 65 feet below the existing ground surface. Based on the previous boring logs drilled in 1974, auger refusal was not encountered to depths of 100 feet below the existing ground surface. However, very stiff Potomac Group sands and clays were encountered as borings were extended deeper.

4.2 Adding Loads to the Existing Foundations

The structural engineer, McMullan and Associates anticipates that the additional loading will require an increase of the soil bearing capacity of between 5 to 11 percent. We have reviewed the as-built foundation drawings. Based on the original design drawings, the foundations were designed for residential townhouses or similar lightly loaded residential structures to be constructed where the existing top parking deck is currently utilized to park vehicles. The structural field investigation will verify the structural integrity of the parking garage structure and is submitted under separate cover.

Based on our soil investigation data and review of the original design drawings, we estimate that the foundations are bearing in the Potomac Group dense sands and stiff clays. Based on our initial analysis, we believe that the existing foundations are capable of supporting the additional loads from the proposed one-level of construction on the parking deck. If additional stories or levels are planned above the 2-story townhouses, additional analysis and possible field investigations will need to occur.

5.0 General Limitations

Recommendations contained in this report are based upon the data obtained from the relatively limited number of test borings. This report does not reflect conditions that may occur between the points investigated, or between sampling intervals in test borings. The nature and extent of variations between test borings and sampling intervals may not become evident until the course of construction. Therefore, it is essential that on-site observations of subgrade conditions be performed during the construction period

to determine if re-evaluation of the recommendations in this report must be made. It is critical to the successful completion of this project that GeoCapitol be retained during construction to observe the implementation of the recommendations provided herein.

This report has been prepared to aid in the evaluation of the site and to assist your office and the design professionals in the design of this project. It is intended for use with regard to the specific project as described herein. Changes in proposed construction, grading plans, structural loads, etc. should be brought to our attention so that we may determine any effect on the recommendations presented herein.

An allowance should be established for additional costs that may be required for foundation and earthwork construction as recommended in this report. Additional costs may be incurred for various reasons including wet fill materials, soft subgrade conditions, unexpected groundwater problems, rock excavation, etc.

This report should be made available to bidders prior to submitting their proposals to supply them with facts relative to the subsurface conditions revealed by our investigation and the results of analyses and studies that have been performed for this project. In addition, this report should be given to the successful contractor and subcontractors for their information only.

We recommend the project specifications contain the following statement: "A geotechnical soils investigation report has been prepared for this project by GeoCapitol Engineering LLC This report is for informational purposes only and should not be considered part of the contract documents. The opinions expressed in this report are those of the geotechnical engineer and represent their interpretation of the subsoil conditions, tests and results of analyses that they performed. Should the data contained in this report not be adequate for the contractor's purposes, the contractor may make their own investigations, tests and analyses prior to bidding."

This report was prepared in accordance with generally accepted geotechnical engineering practices. No warranties, expressed or implied, are made as to the professional services included in this report.



We appreciate the opportunity to be of service for this project. Please contact the undersigned if you require clarification of any aspect of this report.

Sincerely,

GEOCAPITOL ENGINEERING LLC



Ashley Hogan, PG
Senior Geologist

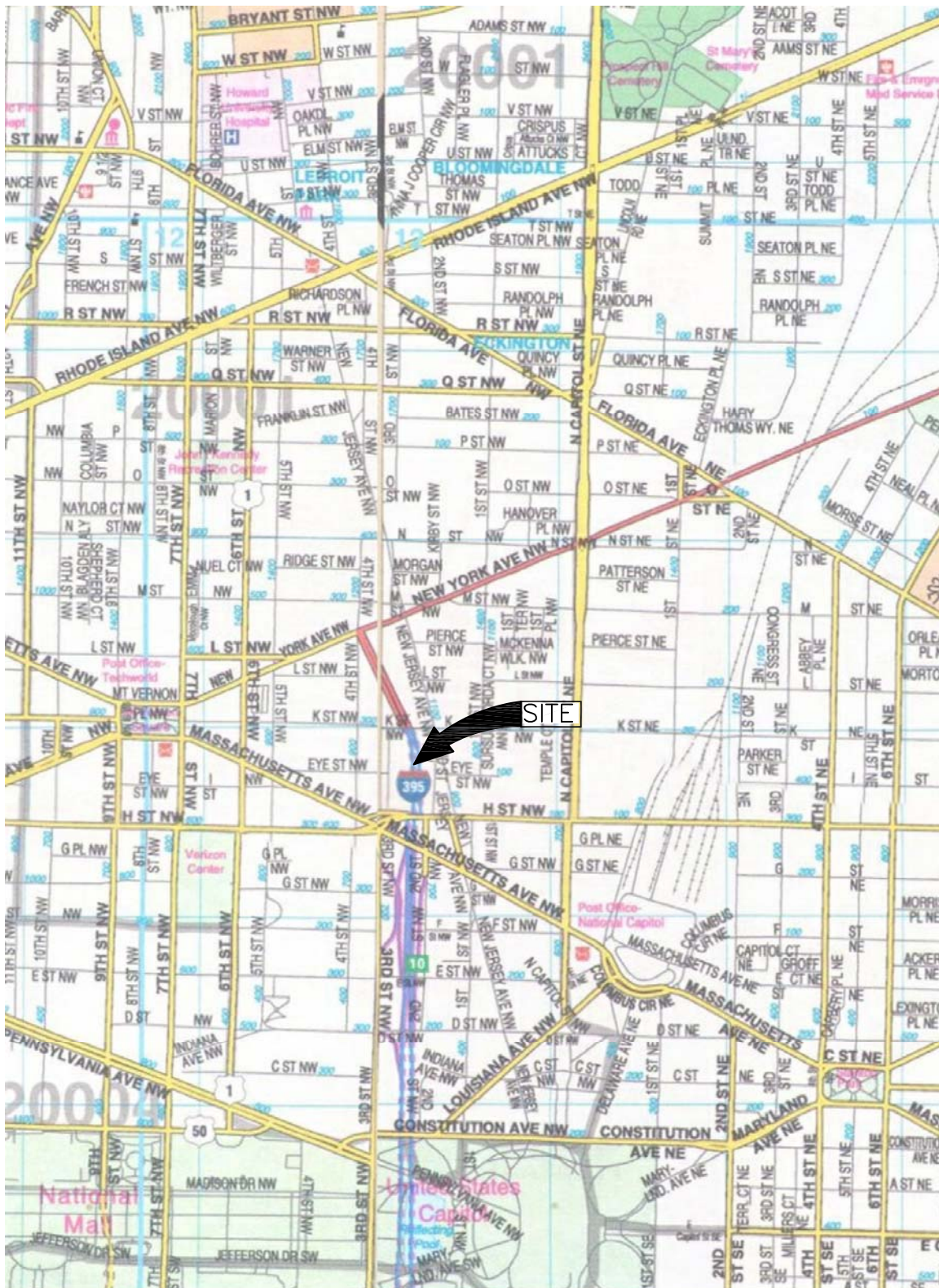


Daniel F. Gradishar, PE
Principal

Copy: Ms. Polina Bakhtearov (email)

ST/AH/DG/kf

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**GeoCapitol
Engineering LLC**

4545 42nd Street NW, Suite 307
Washington, DC 20016

(202) 375-7900

200 K STREET NW PARKING DECK
WASHINGTON, DC 20001

SITE VICINITY
MAP

Scale:
N.T.S.

Fig.

Date:
SEPT. 2016

Checked By:
D.F.G.

Project No.:
DC16024

1

Appendix A

Subsurface Investigation

Subsurface Investigation Procedures (1 page)

Identification of Soil (1 page)

Test Boring Notes (1 page)

Test Boring Logs (3 pages)

Previous Test Boring Logs (24 pages)

Boring Location Plan, Figure 2, (1 page)

Old Boring Location Plan, Figure 3 (1 page)

Subsurface Investigation Procedures

1. Test Borings – Hollow Stem Augers

The borings are advanced by turning an auger with a center opening of 2¼ inches. A plug device blocks off the center opening while augers are advanced. Cuttings are brought to the surface by the auger flights. Sampling is performed through the center opening in the hollow stem auger, by standard methods, after removal of the plug. Usually, no water is introduced into the boring using this procedure.

2. Standard Penetration Tests

Standard penetration tests are performed by driving a 2 inch O.D., 1-¾ inch I.D. sampling spoon with a 140-pound hammer falling 30 inches, according to ASTM D-1586. After an initial 6 inches penetration to assure the sampling spoon is in undisturbed material, the number of blows required to drive the sampler an additional 12 inches is generally taken as the N value. In the event 30 or more blows are required to drive the sampling spoon the initial 6 inch interval, the sampling spoon is driven to a total penetration resistance of 100 blows or 18 inches, whichever occurs first.

3. Test Boring Stakeout

The test boring stakeout was provided by GeoCapitol personnel using available site plans. Ground surface elevations were estimated from topographic information contained on the site plan provided to us and should be considered approximate. If the risk related to using approximate boring locations and elevations is unacceptable, we recommend an as-drilled survey of boring locations and elevations be completed by a licensed surveyor.

Identification of Soil

I. DEFINITION OF SOIL GROUP NAMES		ASTM D-2487	Symbol	Group Name
Coarse-Grained Soils More than 50% retained on No. 200 sieve	Gravels More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels Less than 5% fines	<i>GW</i>	WELL GRADED GRAVEL
			<i>GP</i>	POORLY GRADED GRAVEL
		Gravels with Fines More than 12% fines	<i>GM</i>	silty GRAVEL
			<i>GC</i>	clayey GRAVEL
	Sands 50% or more of coarse fraction passes No. 4 sieve	Clean Sands Less than 5% fines	<i>SW</i>	WELL GRADED SAND
			<i>SP</i>	POORLY GRADED SAND
		Sands with fines More than 12% fines	<i>SM</i>	silty SAND
			<i>SC</i>	clayey SAND
Fine-Grained Soils 50% or more passes the No. 200 sieve	Silts and Clays Liquid Limit less than 50	Inorganic	<i>CL</i>	LEAN CLAY
			<i>ML</i>	SILT
		Organic	<i>OL</i>	ORGANIC CLAY
				ORGANIC SILT
	Silts and Clays Liquid Limit 50 or more	Inorganic	<i>CH</i>	FAT CLAY
			<i>MH</i>	ELASTIC SILT
		Organic	<i>OH</i>	ORGANIC CLAY
				ORGANIC SILT
Highly Organic Soils	Primarily organic matter, dark in color, and organic odor	<i>PT</i>	PEAT	

II. DEFINITION OF MINOR COMPONENT PROPORTIONS

Minor Component	Approximate Percentage of Fraction by Weight
Gravelly, Sandy (adjective)	30% or more coarse grained
Sand, Gravel (with)	15% to 29% coarse grained
Silt, Clay (with)	5% to 12% fine grained

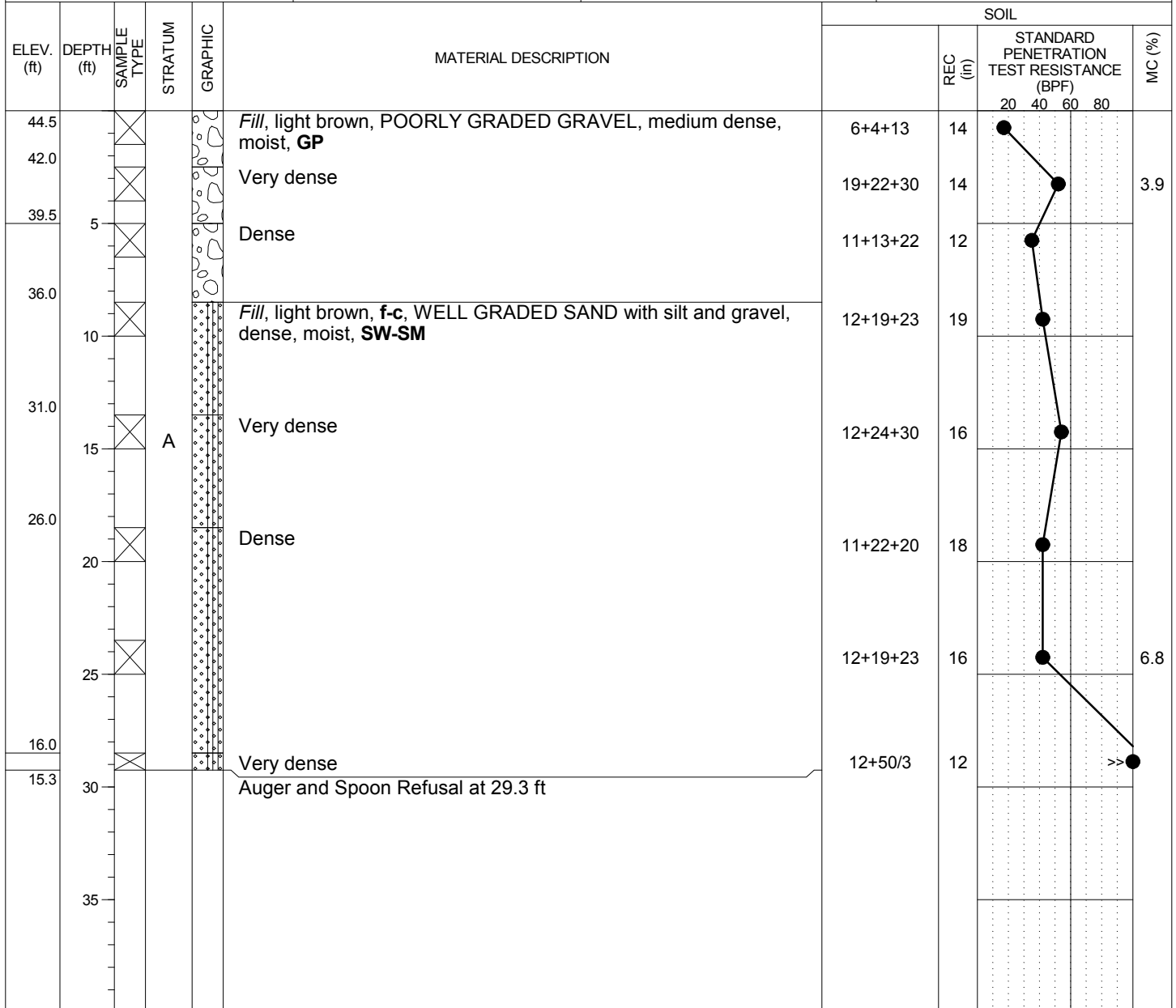
III. GLOSSARY OF MISCELLANEOUS TERMS

SYMBOLS	Unified Soil Classification Symbols are shown above as group symbols. Use "A" Line Chart for laboratory identification. Dual symbols are used for borderline classification.
BOULDERS & COBBLES	Boulders are considered pieces of rock larger than 12 inches, while cobbles range from 3 to 12 inches.
WEATHERED ROCK	Residual rock material with a standard penetration test (SPT) resistance between 60 blows per foot.
ROCK/SPOON REFUSAL	Rock material with a standard penetration test (SPT) resistance of 50 blows for 1 inch.
ROCK FRAGMENTS	Angular pieces of rock which have separated from original vein or strata and are present in a soil matrix. Only used in residual soils.
QUARTZ	A hard silicate mineral often found in residual soils. Only used when describing residual soils.
CEMENTED SAND	Usually localized rock-like deposits within a soil stratum composed of sand grains cemented by calcium carbonate, iron oxide, or other minerals. Commonly encountered in Coastal Plain sediments, primarily in the Potomac Group sands (Kps).
MICACEOUS	A term used to describe soil that "glitters" or is shiny. Most commonly encountered in fine-grained soils.
ORGANIC MATERIALS (Excluding Peat)	Topsoil - Surface soils that support plant life and contain organic matter. Lignite - Hard, brittle decomposed organic matter with low fixed carbon content (a low grade of coal).
FILL	Man made deposit containing soil, rock, and other foreign matter.
CONTAINS	This is used when a fill deposit contains a secondary component that does not apply to a USCS classification. Only used for fill deposits
WITH	This is used when a residual soil contains a secondary component that does not contribute to its USCS classification. Only used for natural soils.
PROBABLE FILL LAYERS	Soils which contain no visually detected foreign matter but which are suspect with regard to origin. ½ to 12 inch seam of minor soil component.
COLOR	Two most predominant colors present should be described.
MOISTURE CONDITIONS	Wet, moist, or dry to indicate visual appearance of specimen.
f-m-c	Fine-medium-coarse

Test Boring Notes

1. Classification of soil is by visual inspection and is in accordance with the Unified Soil Classification System.
2. Estimated groundwater levels are indicated on the logs. These are only estimates from available data and may vary with precipitation, porosity of soil, site topography, etc.
3. Sampling data presents standard penetrations for 6-inch intervals or as indicated with graphic representations adjacent to the sampling data.
4. The logs and related information depict subsurface conditions at the specific locations and at the particular time when drilled. Soil conditions at other locations may differ from conditions occurring at the test locations. Also, the passage of time may result in a change in the subsurface conditions at the test locations.
5. The stratification lines represent the approximate boundary between soil types as determined in the sampling operation. Some variation may be expected vertically between samples taken. The soil profile, groundwater level observations and penetration resistances presented on the logs have been made with reasonable care and accuracy and must be considered only an approximate representation of subsurface conditions to be encountered at the particular location.

Parking Deck Engineering Analysis		LOGGED BY: S. Tafesse	B-1
LOCATION: 200 K Street, NW, Washington, DC		DRILLING CONTRACTOR: Connelly & Associates Inc.	
OWNER/CLIENT: DC Deputy Mayor for Planning & Econ. Development		DRILLER: J. Leatherman	DATES DRILLED: 7/15/16 - 7/15/16
PROJECT NUMBER: DC16024	GROUND SURFACE ELEVATION (ft): 44.5	DRILLING METHOD: Automatic hammer 2.25"	OFFSET NOTES:

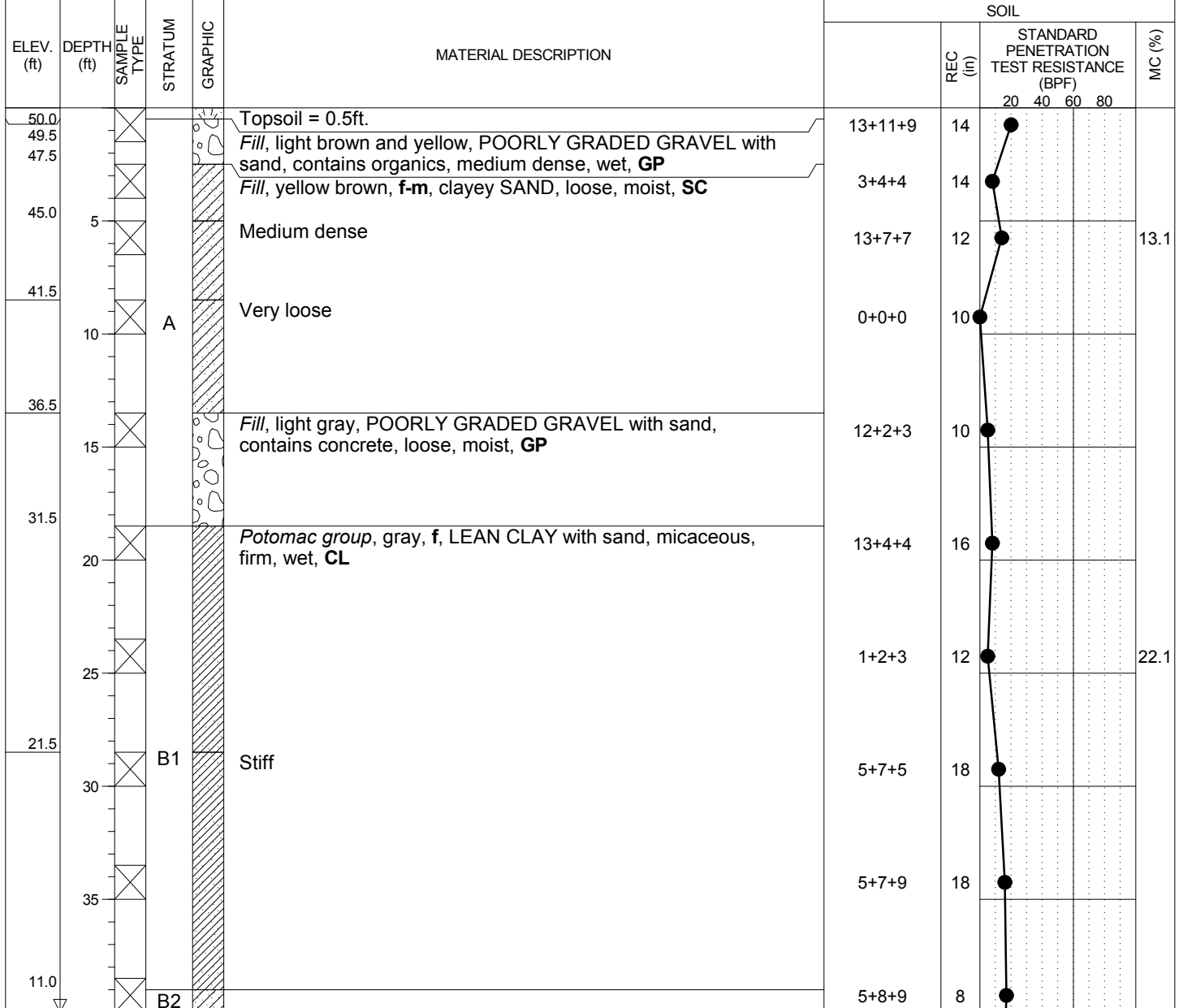


GROUND WATER LEVELS: NOT ENCOUNTERED DURING DRILLING NOT ENCOUNTERED UPON COMPLETION	SAMPLE TYPES:  Split Spoon
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REMARKS: Refusal at 29.3 feet

BOREHOLE/TEST PIT PARKING DECK ENGINEERING ANALYSIS.GPJ GEOCONCEPTS TEMPLATE 02-12-2015.GDT 8/9/16

Parking Deck Engineering Analysis		LOGGED BY: S. Tafesse	B-2
LOCATION: 200 K Street, NW, Washington, DC		DRILLING CONTRACTOR: Connelly & Associates Inc.	
OWNER/CLIENT: DC Deputy Mayor for Planning & Econ. Development		DRILLER: J. Leatherman	DATES DRILLED: 7/15/16 - 7/15/16
PROJECT NUMBER: DC16024	GROUND SURFACE ELEVATION (ft): 50.0	DRILLING METHOD: Automatic hammer 2.25"	OFFSET NOTES:



GROUND WATER LEVELS: ▽ ENCOUNTERED: <u>40.0</u> ft ELEV. <u>10.0</u> NOT ENCOUNTERED UPON COMPLETION	SAMPLE TYPES: <input checked="" type="checkbox"/> Split Spoon
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REMARKS:

THE STRATIFICATION LINES REPRESENT APPROXIMATE BOUNDARIES. THE TRANSITION MAY BE GRADUAL.

BOREHOLE/TEST PIT PARKING DECK ENGINEERING ANALYSIS.GPJ GEOCONCEPTS TEMPLATE 02-12-2015.GDT 8/9/16

Parking Deck Engineering Analysis	LOGGED BY: S. Tafesse	BORING NUMBER: B-2
LOCATION: 200 K Street, NW, Washington, DC	DRILLING CONTRACTOR: Connelly & Associates Inc.	SHEET 2 OF 2

ELEV. (ft)	DEPTH (ft)	SAMPLE TYPE	STRATUM	GRAPHIC	MATERIAL DESCRIPTION	SOIL		
						REC (in)	STANDARD PENETRATION TEST RESISTANCE (BPF)	MC (%)
					<i>Potomac group, yellow and red, f-m, clayey SAND, medium dense, moist, SC (continued)</i>		20 40 60 80	
	45	X				12+8+7	16	
1.0	50	X			Dark gray, wet	10+11+13	18	
	55	X	B2			4+7+11	18	26.9
-9.2	60	X			<i>Potomac group, light gray, POORLY GRADED SAND, medium dense, moist, SP</i>	5+10+13	18	
-13.5	65	X			Dense	15+21+28	16	
-15.0	65				Bottom of Boring at 65.0 ft			
	70							
	75							
	80							
	85							

GROUND WATER LEVELS: ▽ ENCOUNTERED: <u>40.0</u> ft ELEV. <u>10.0</u> NOT ENCOUNTERED UPON COMPLETION	SAMPLE TYPES: ☒ Split Spoon
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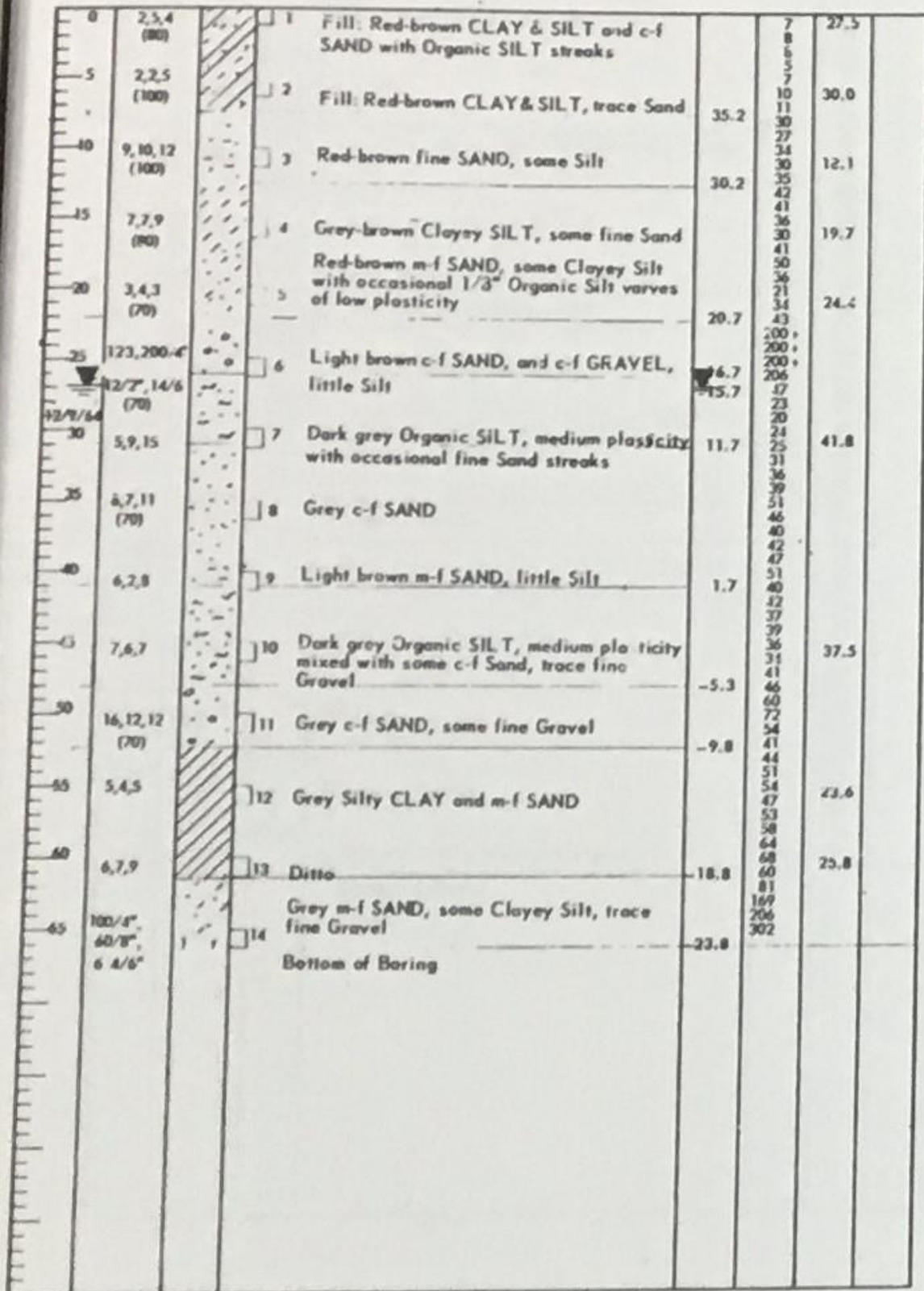
REMARKS:

THE STRATIFICATION LINES REPRESENT APPROXIMATE BOUNDARIES. THE TRANSITION MAY BE GRADUAL.

BOREHOLE/TEST PIT PARKING DECK ENGINEERING ANALYSIS.GPJ GEOCONCEPTS TEMPLATE 02-12-2015.GDT 8/9/16

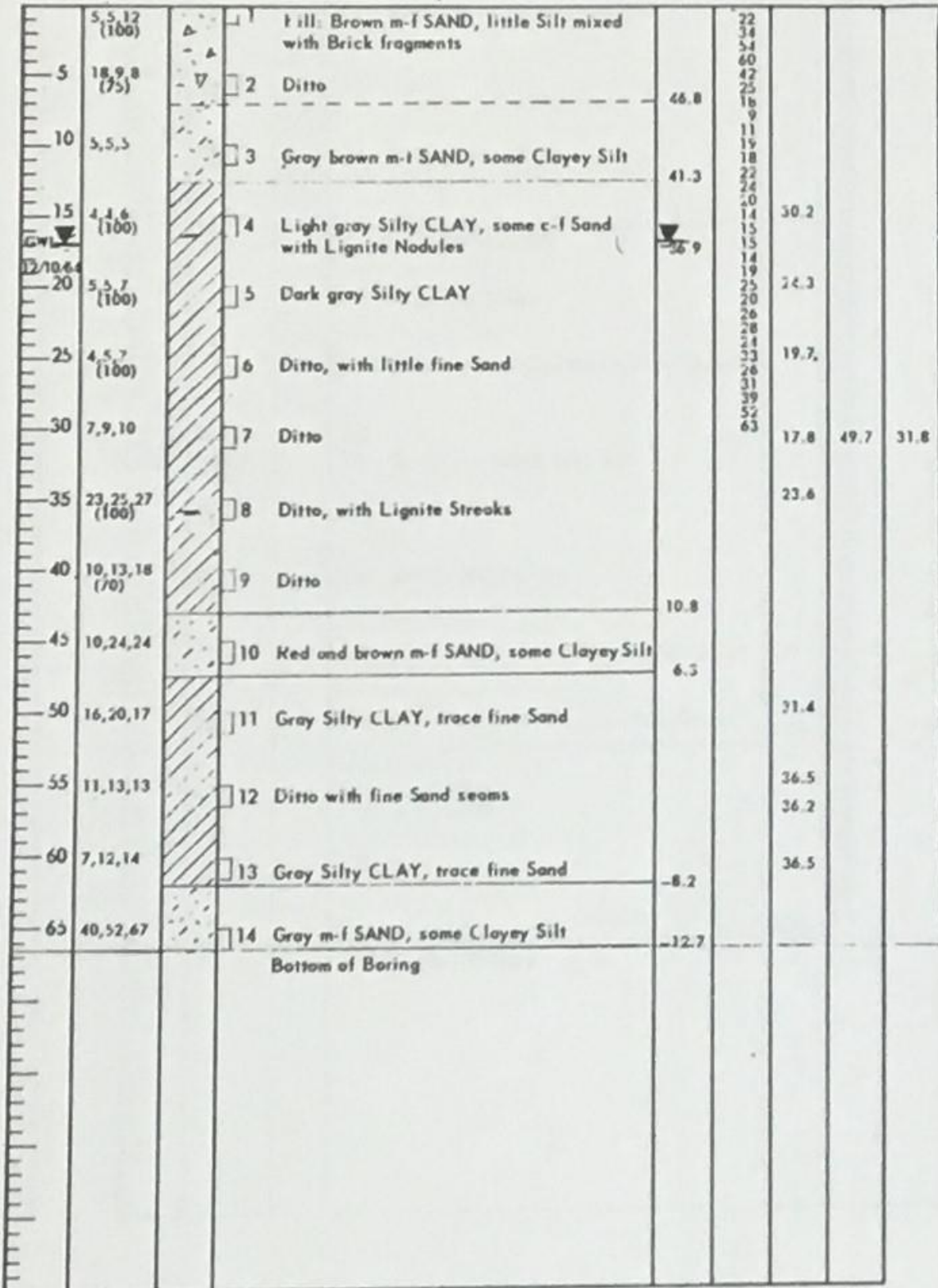
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EL. 42.7



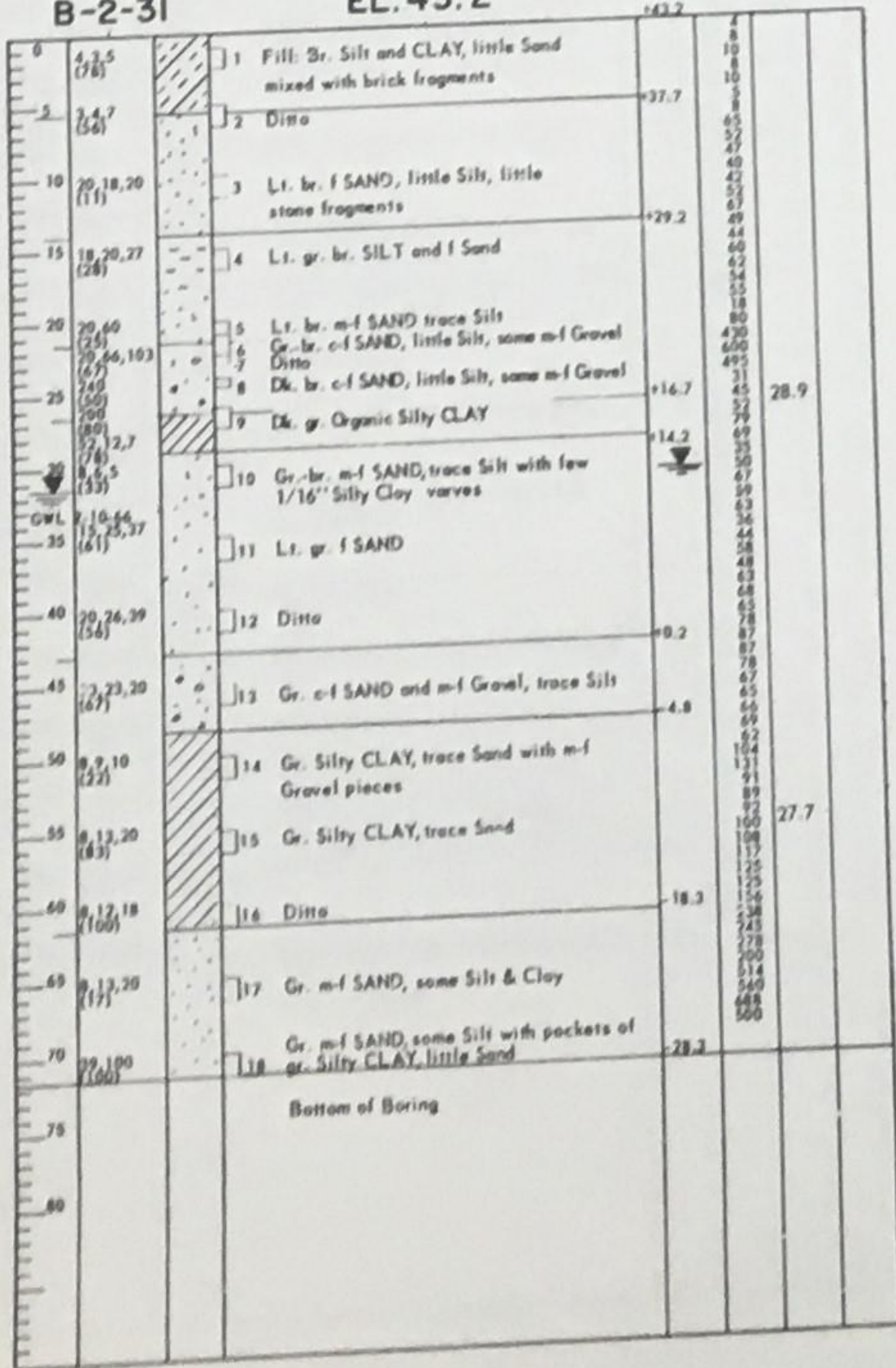
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EL. 53.8



B-2-31

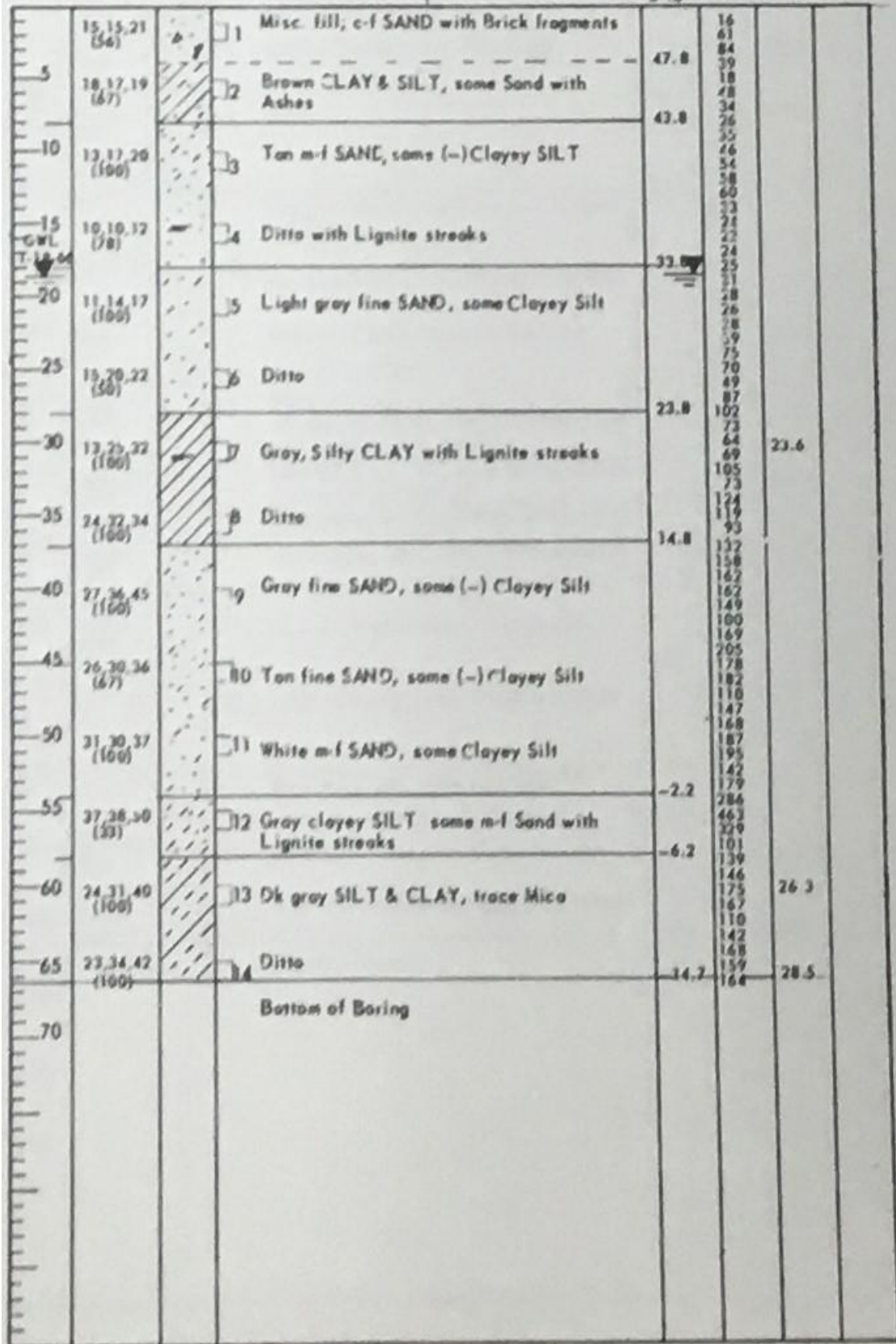
EL. 43.2



B-2-32

EL. 51.8

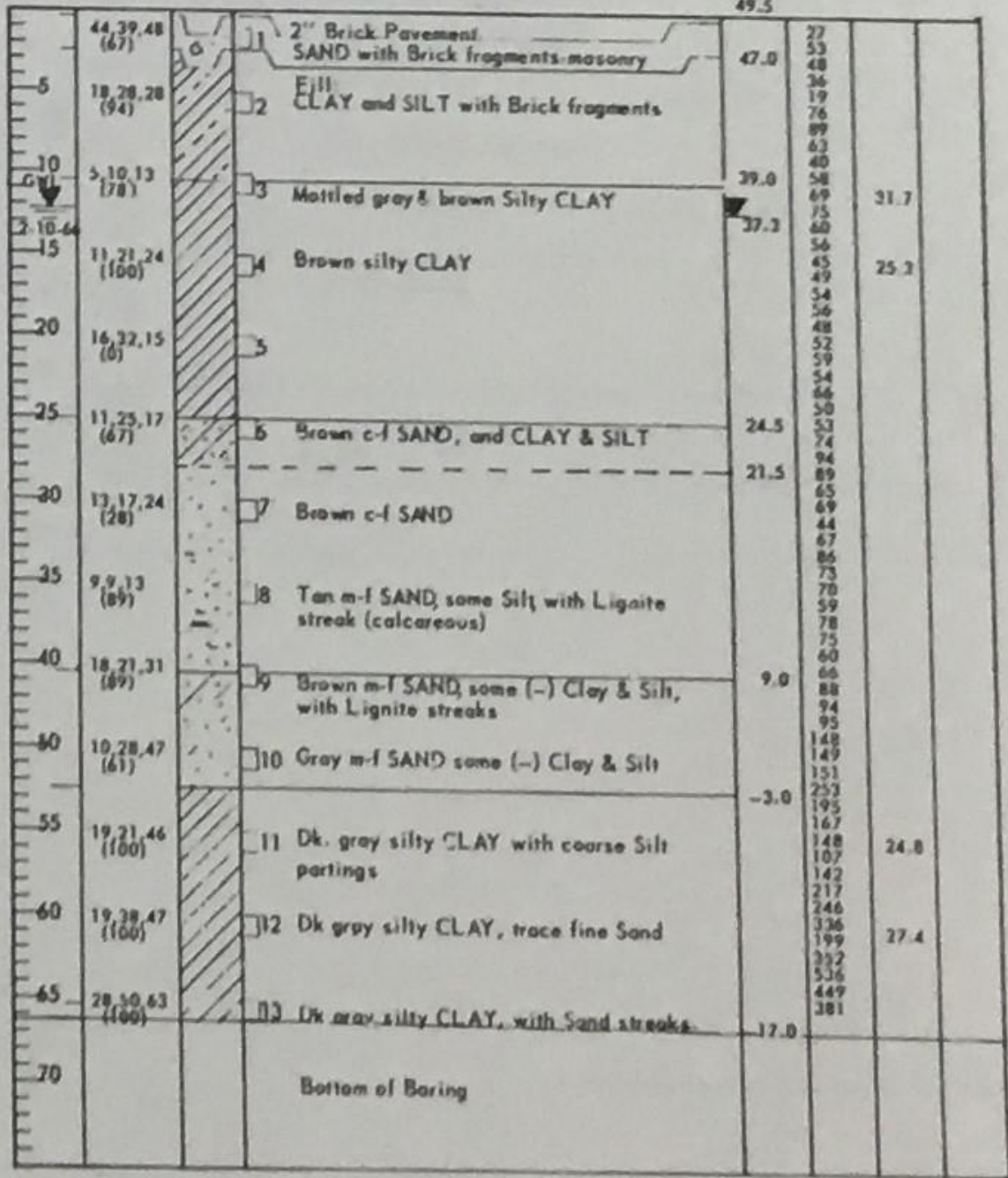
51.8 + 59.2



B-2-33

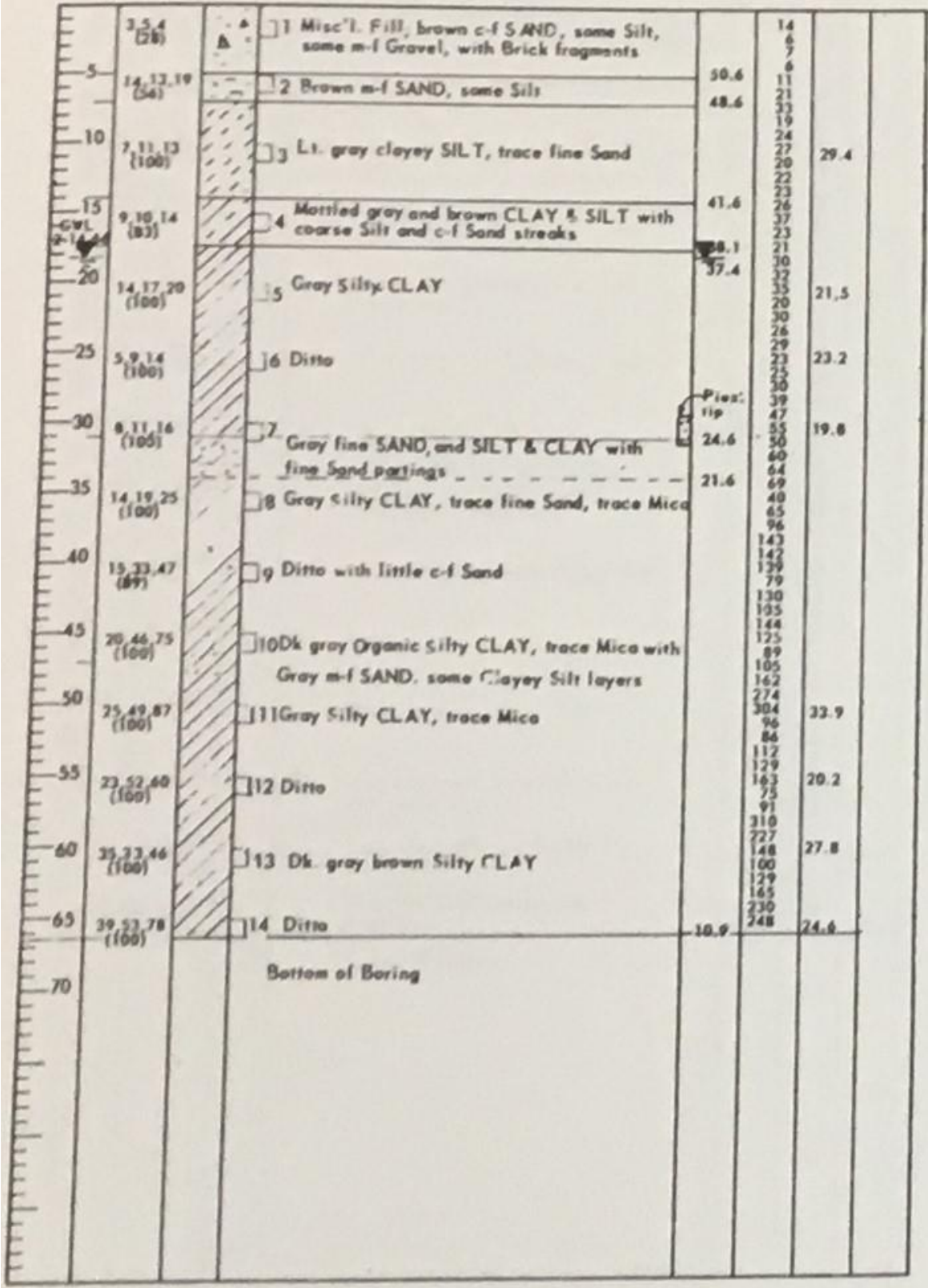
EL. 49.5

49.5



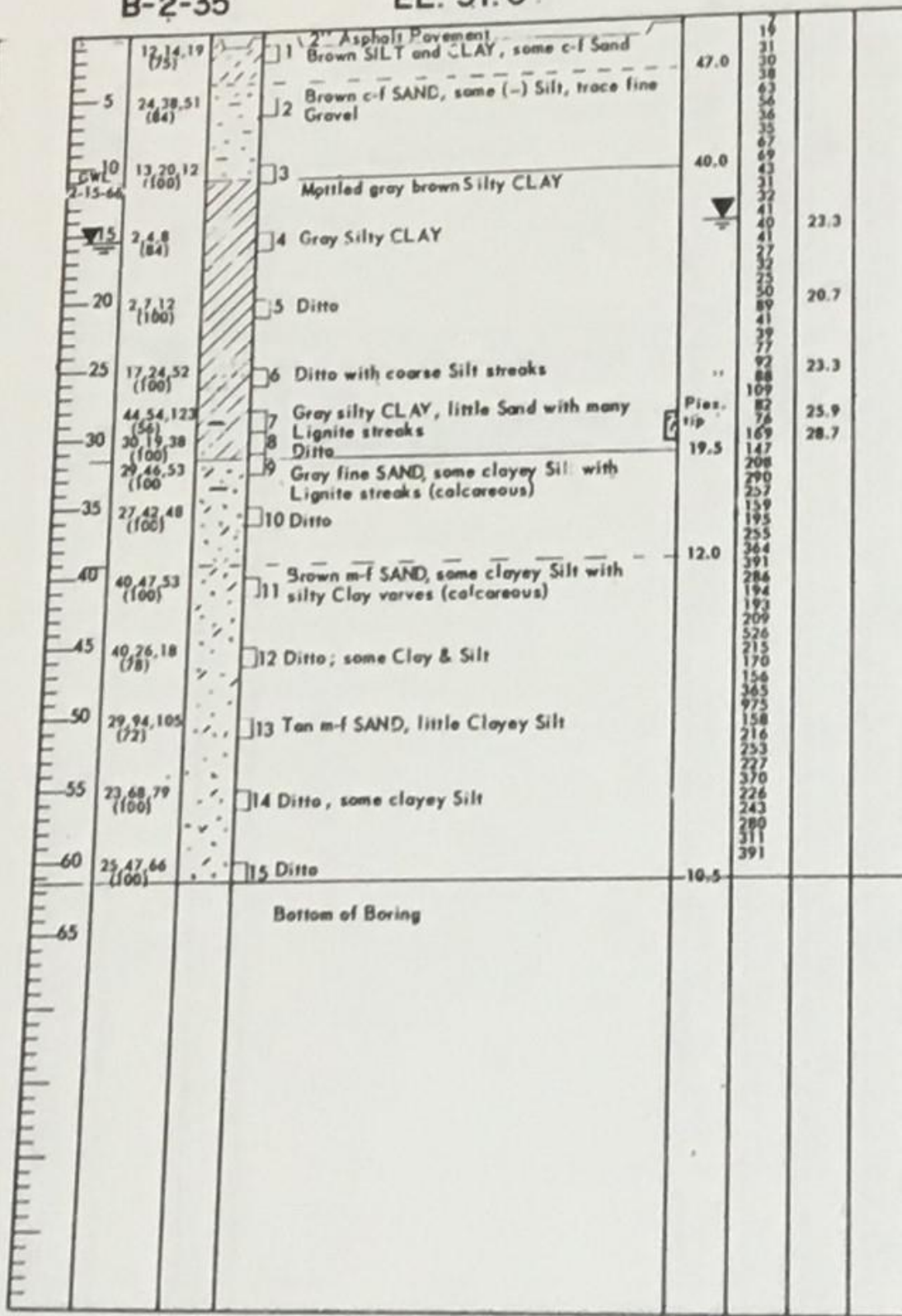
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EL. 55.6



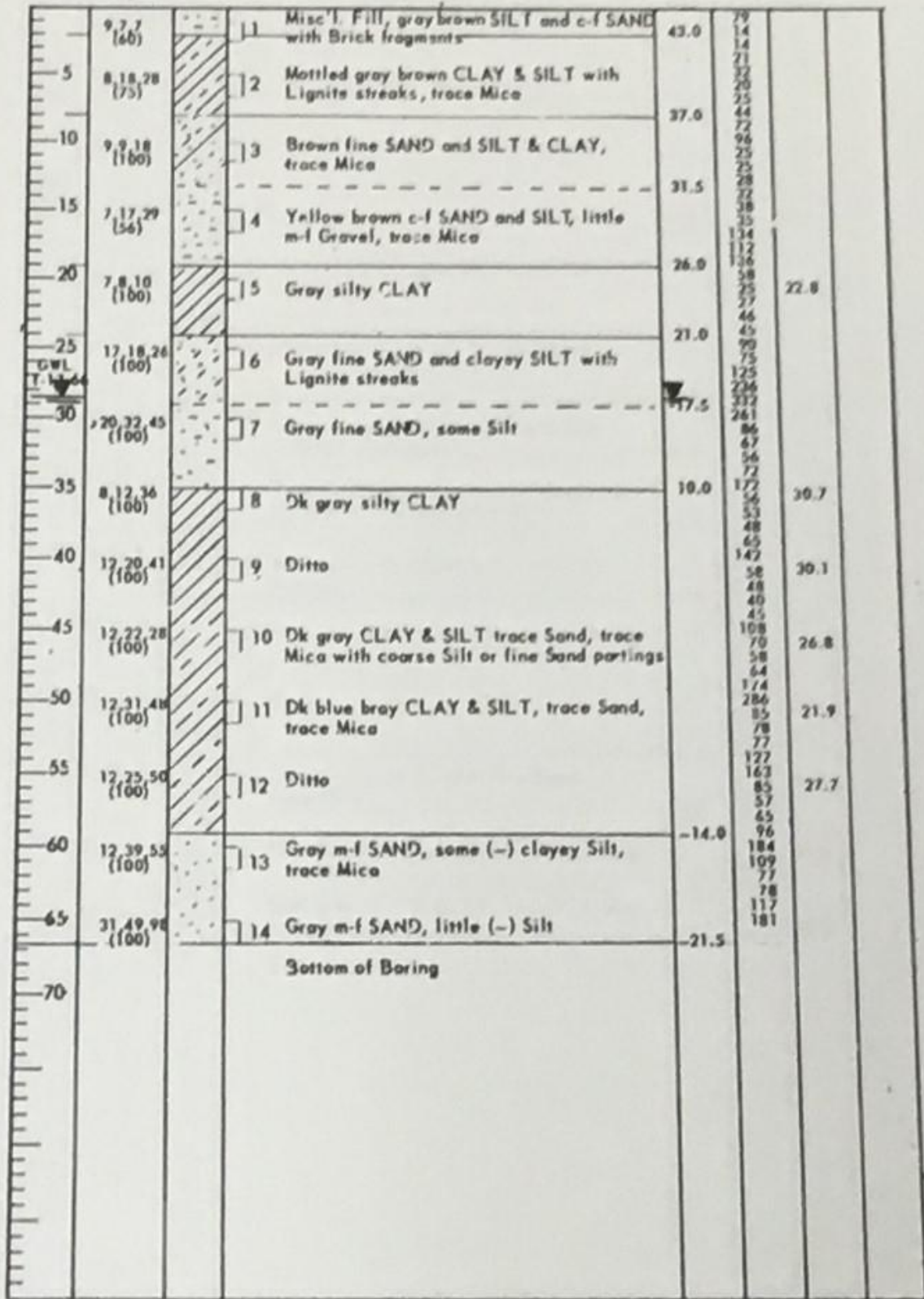
B-2-35

EL. 51.0



B-2-36

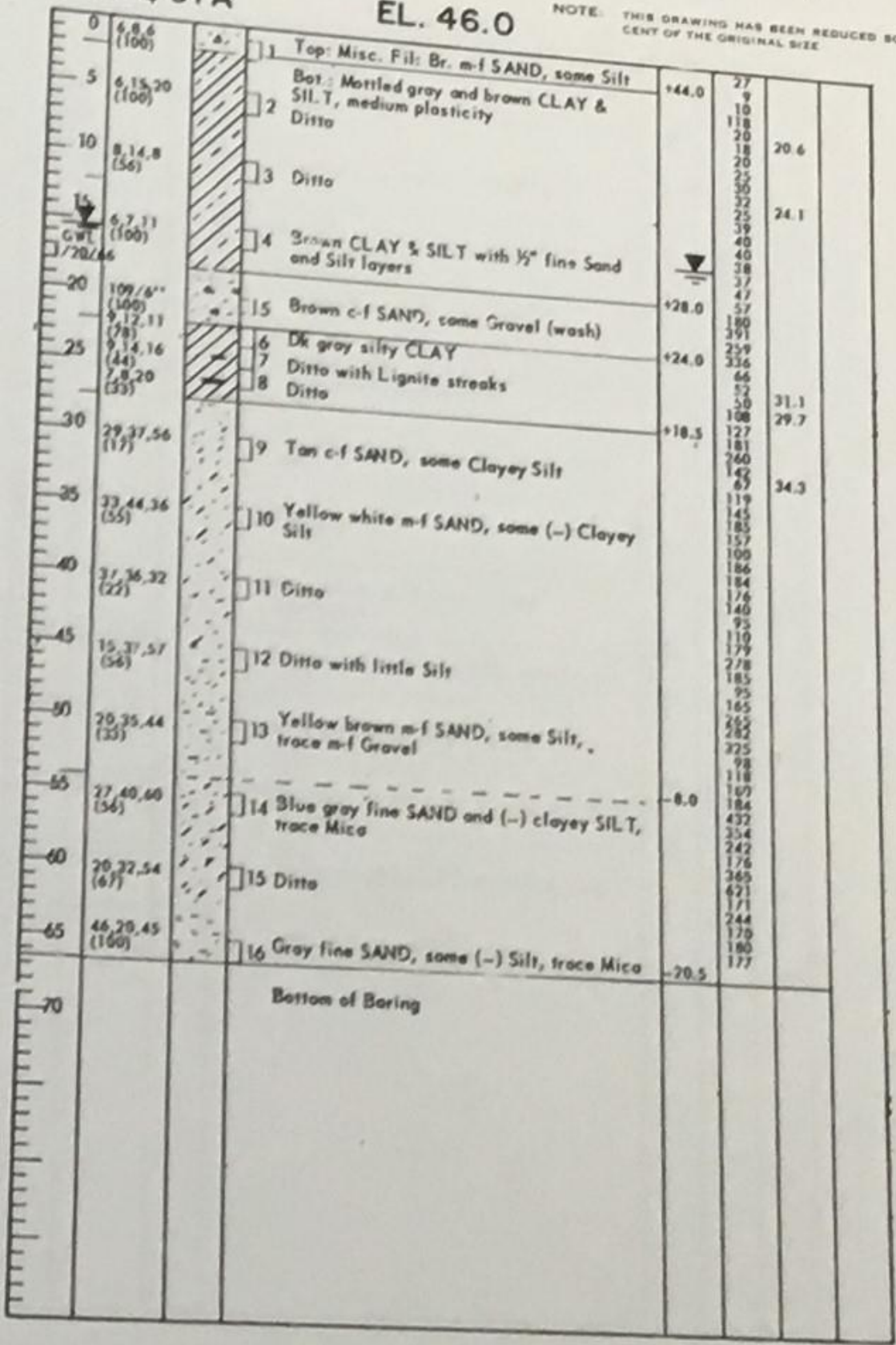
EL. 45.2



B-2-37A

EL. 46.0

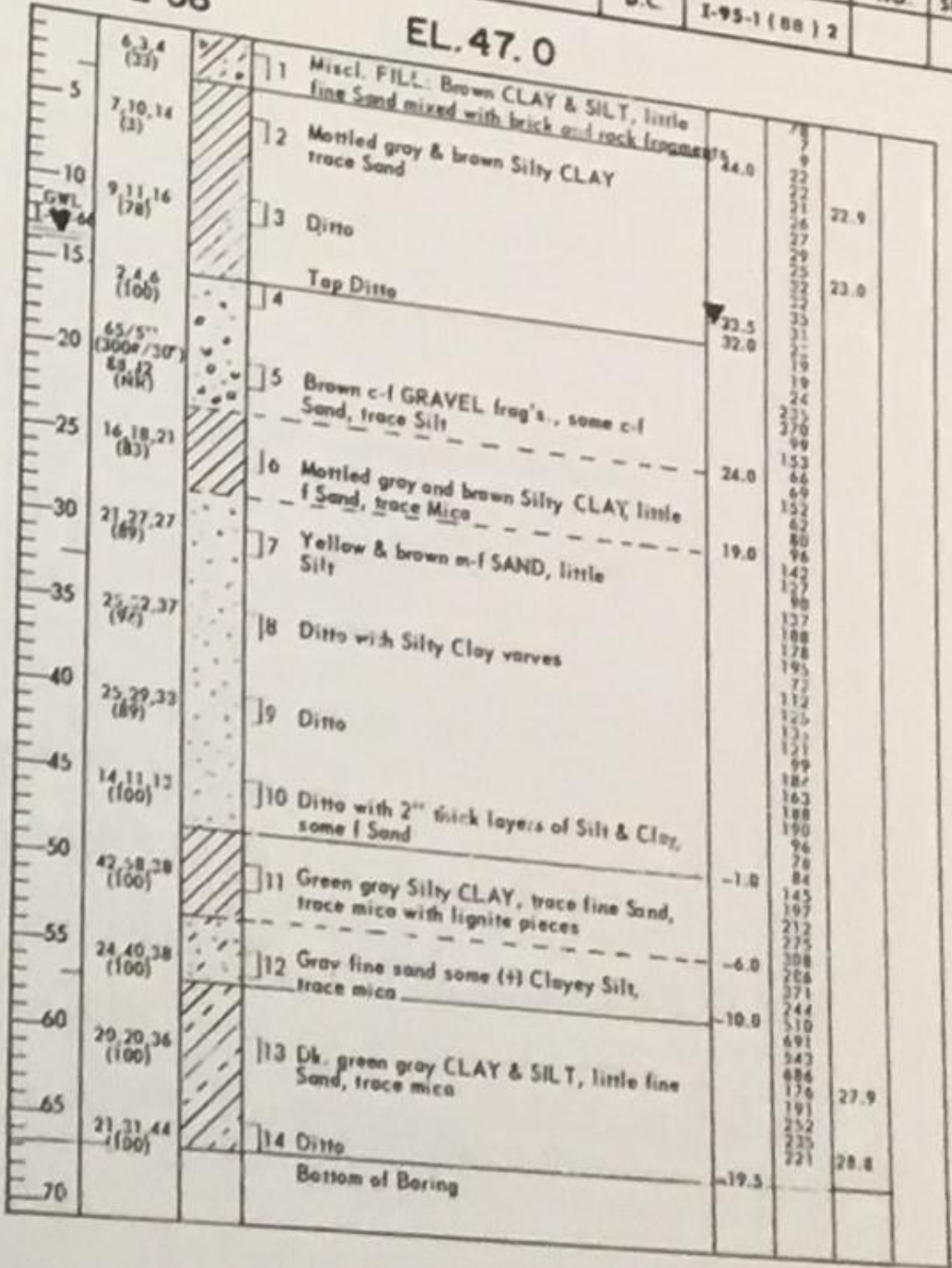
NOTE: THIS DRAWING HAS BEEN REDUCED 80 PERCENT OF THE ORIGINAL SIZE



B-2-38

EL. 47.0

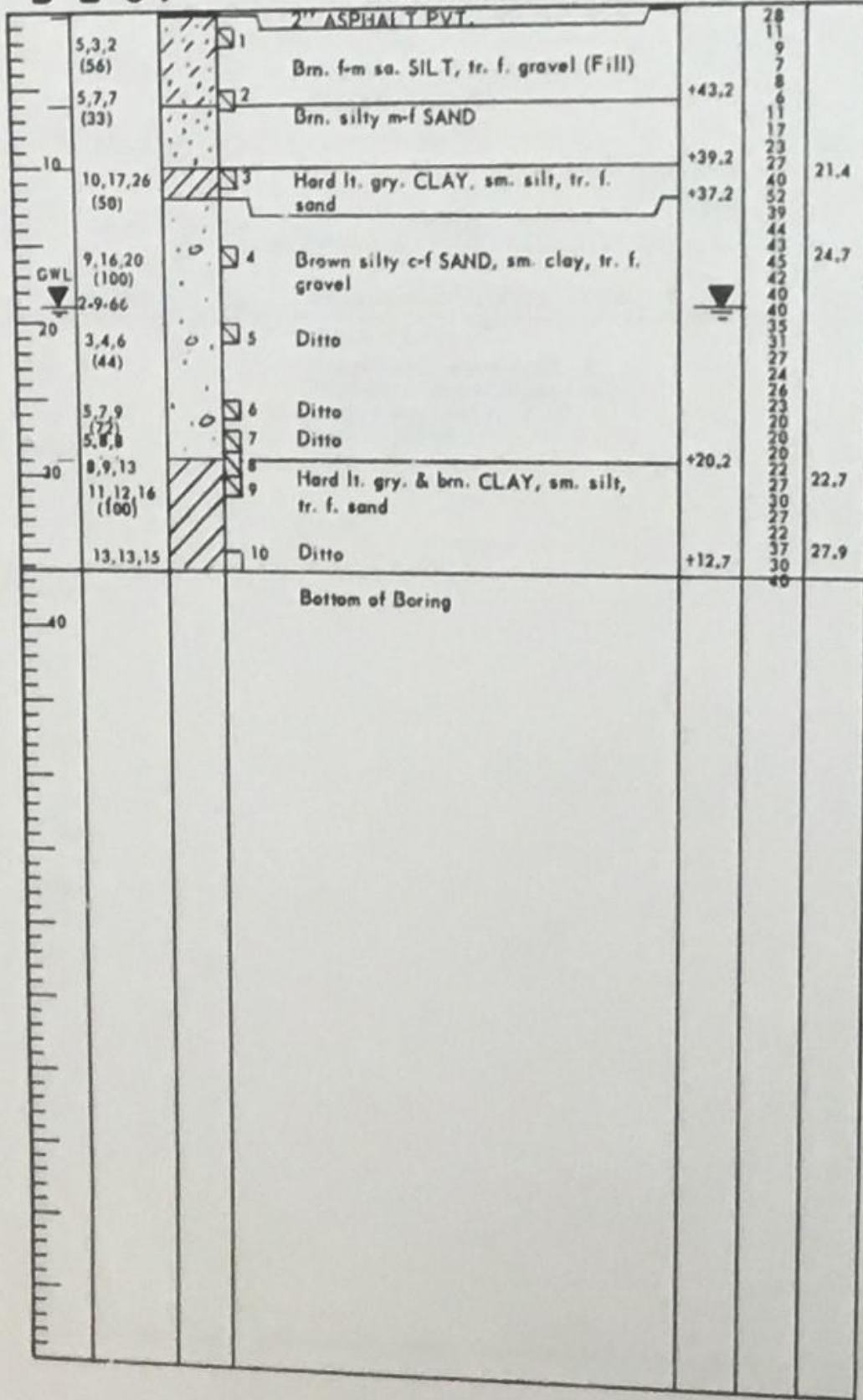
03-51	D.C.	PROJECT NO. I-95-1 (88) 2	SHEET NO.	T. 51
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B-2-54

EL. 49.2

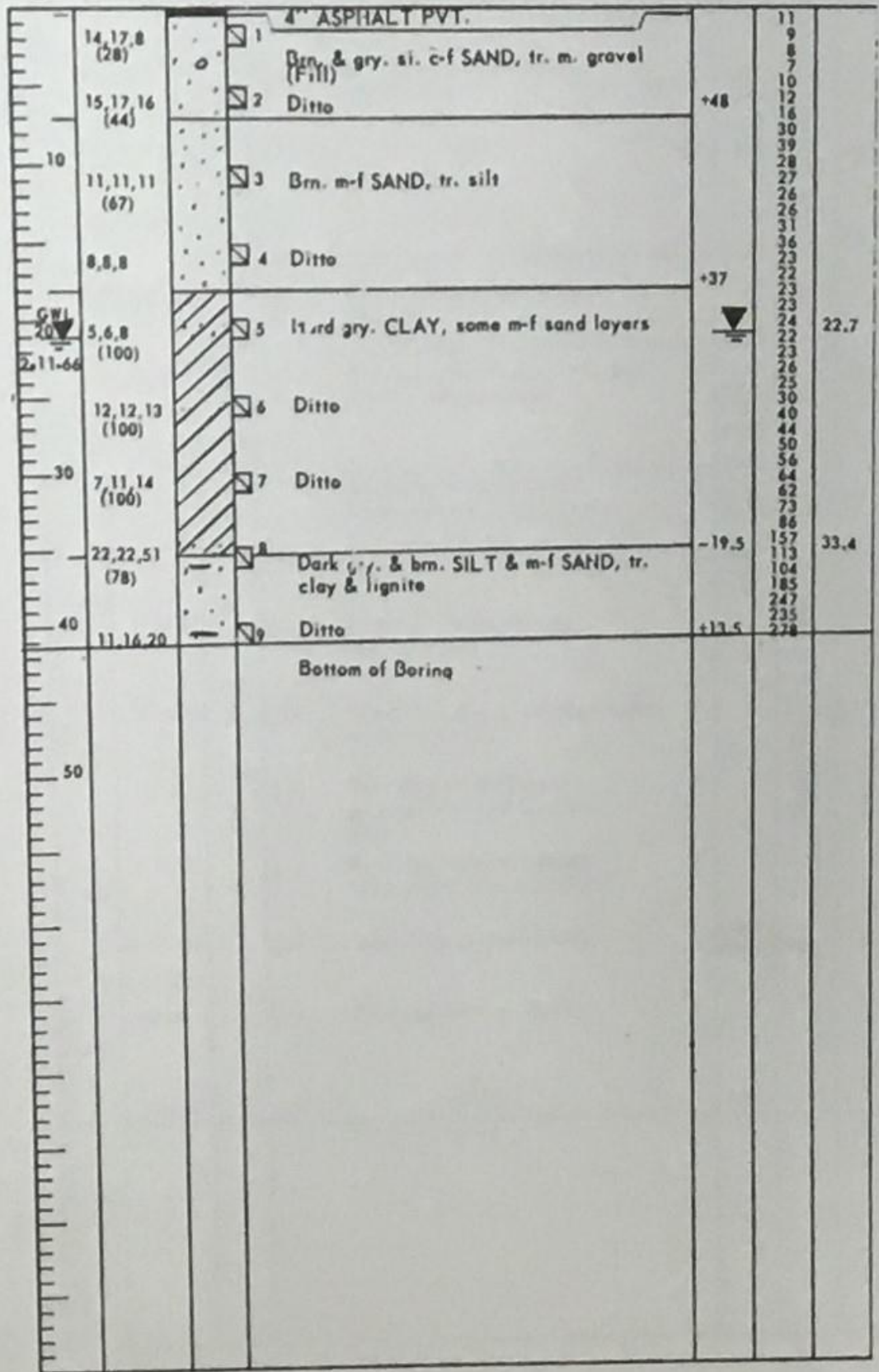
+49.2



B-2-55

EL. 55.0

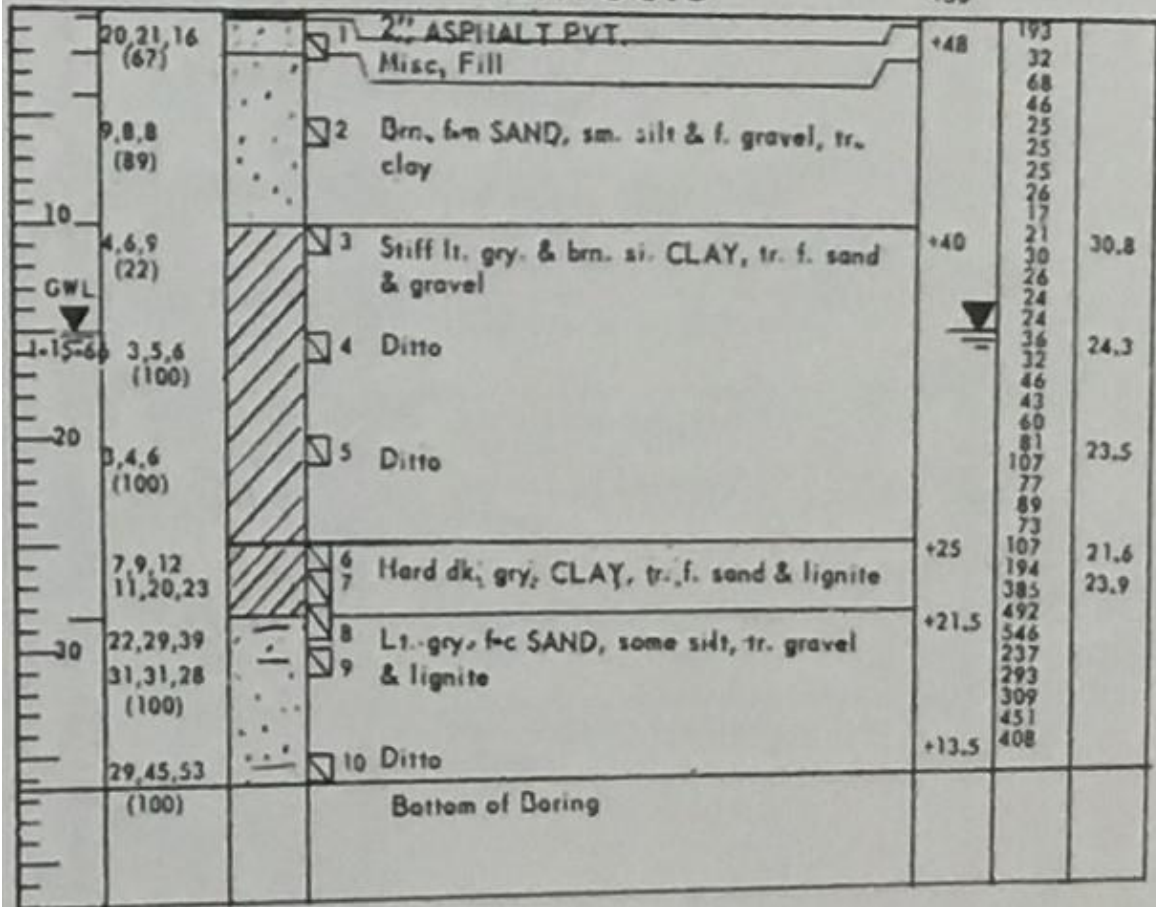
+55



B-2-56

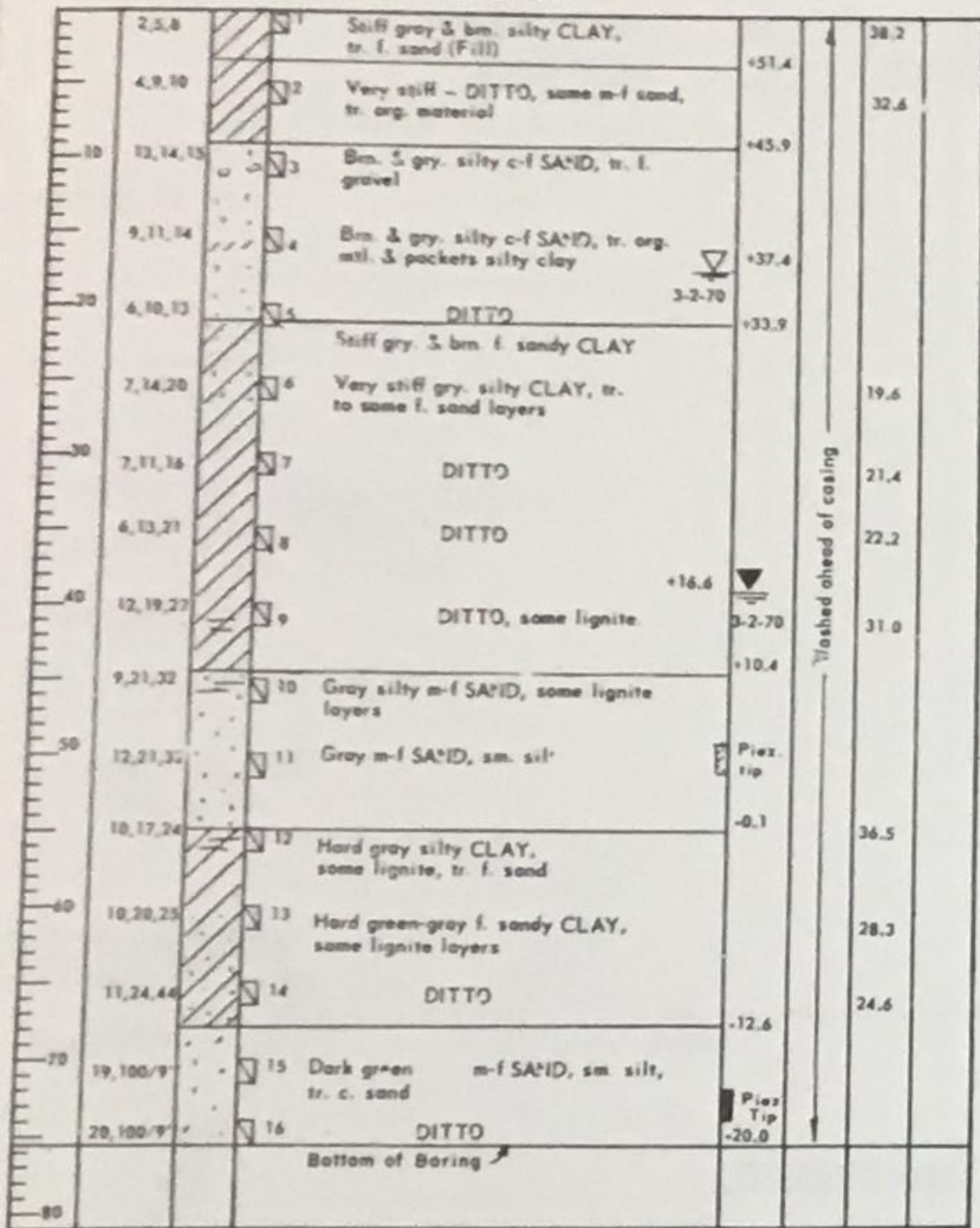
EL. 50.0

+50



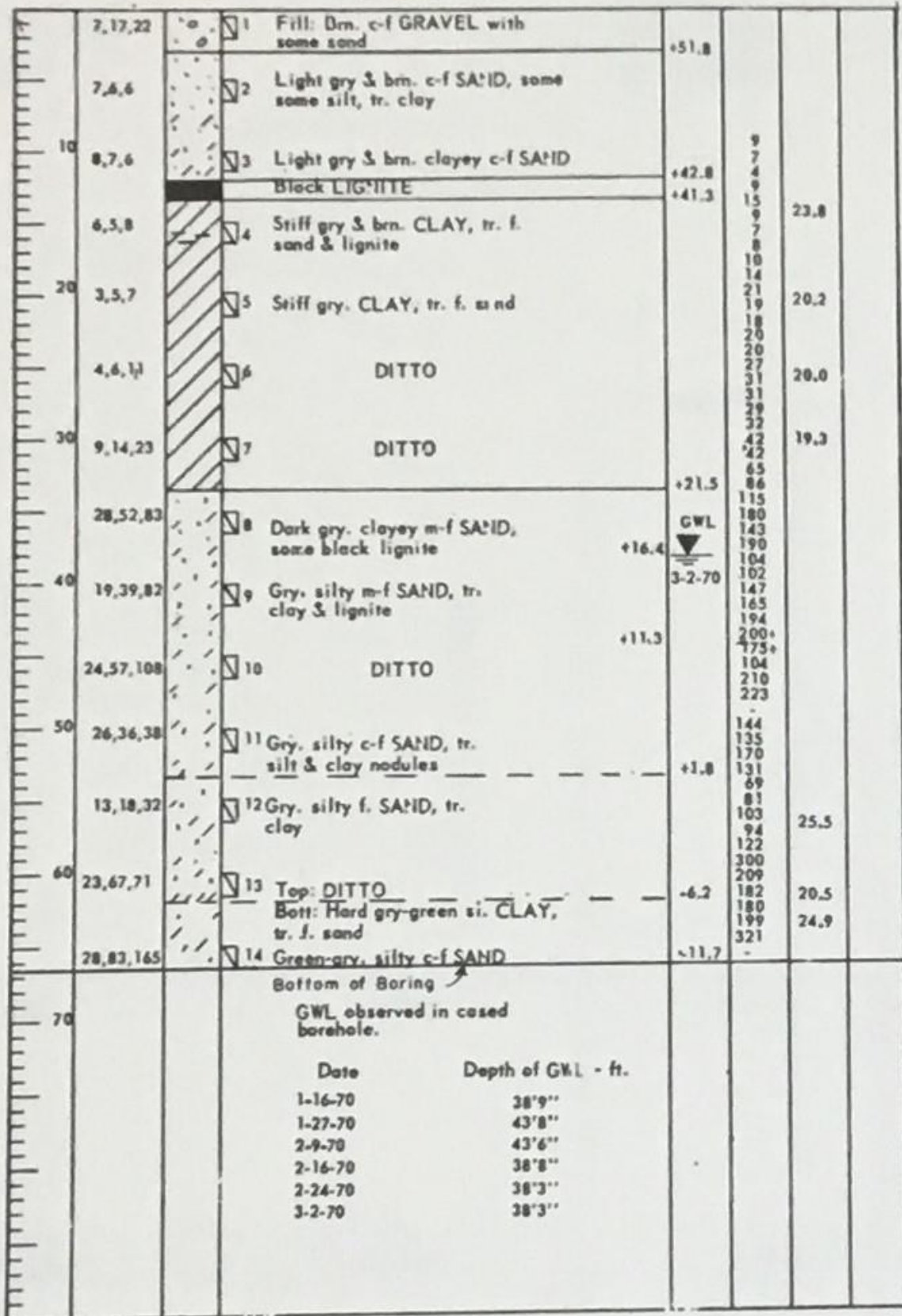
B-2T-1

EL. 54.9



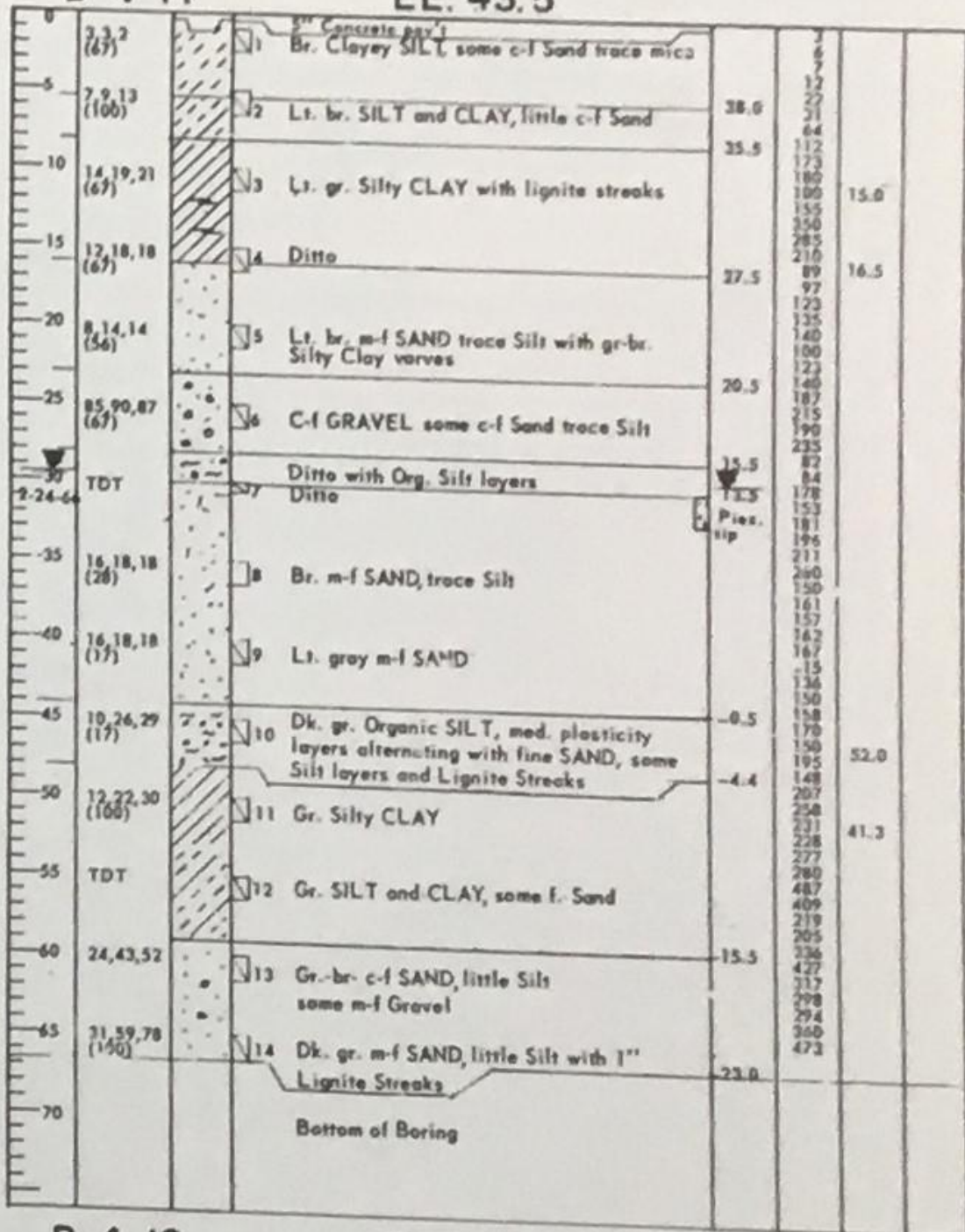
B-2T-2

EL. 54.8



B-4-14

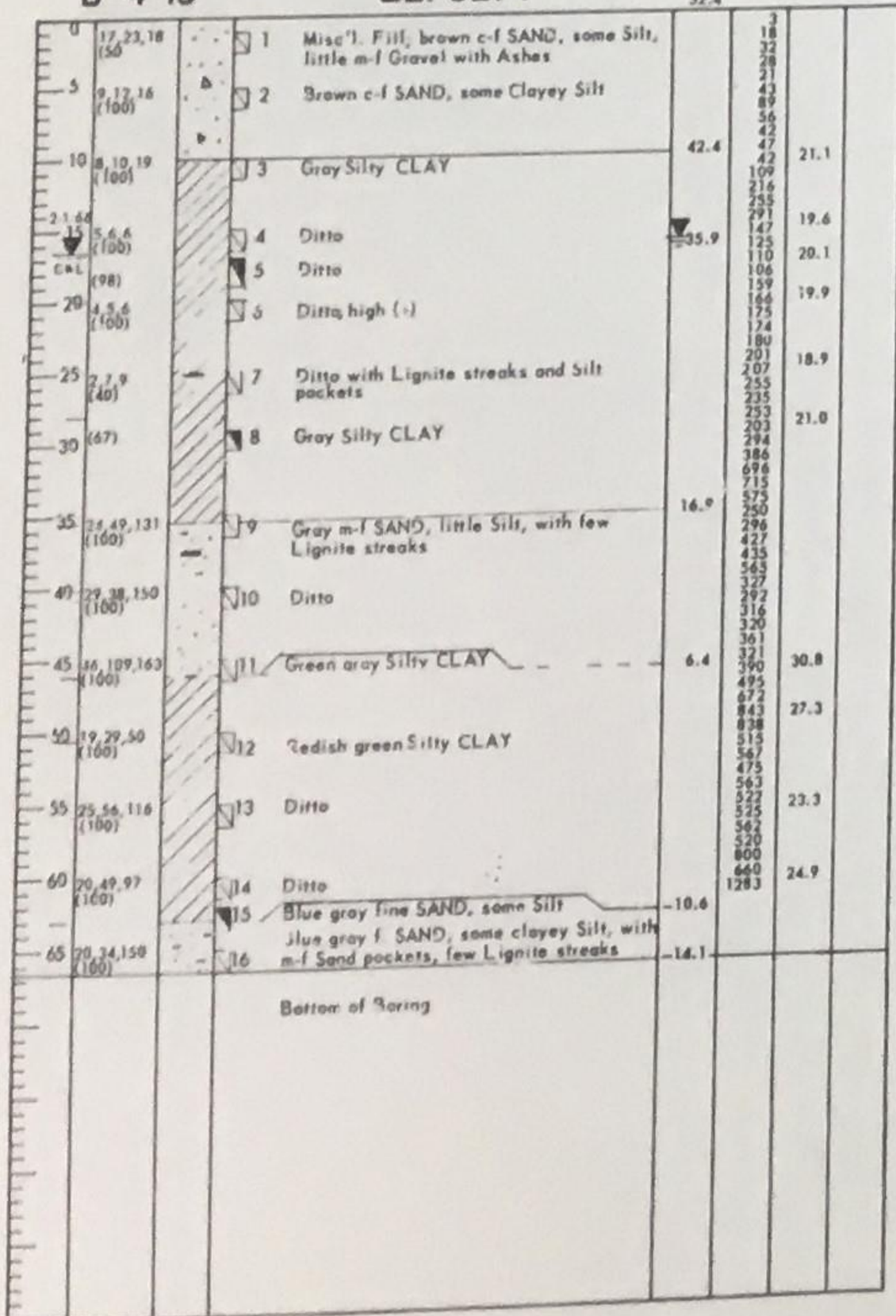
EL. 43.5



B-4-15

EL. 52.4

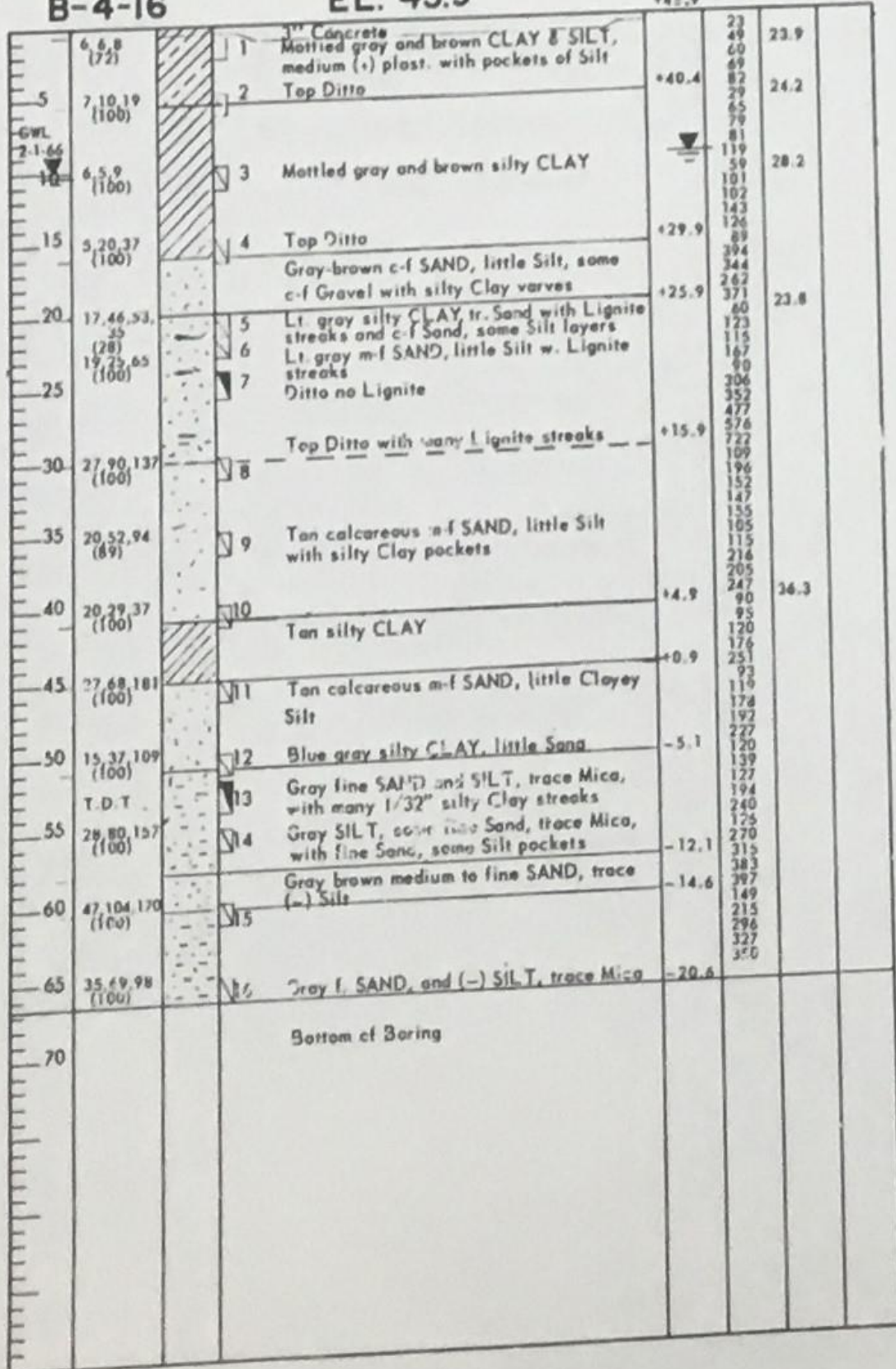
52.4



B-4-16

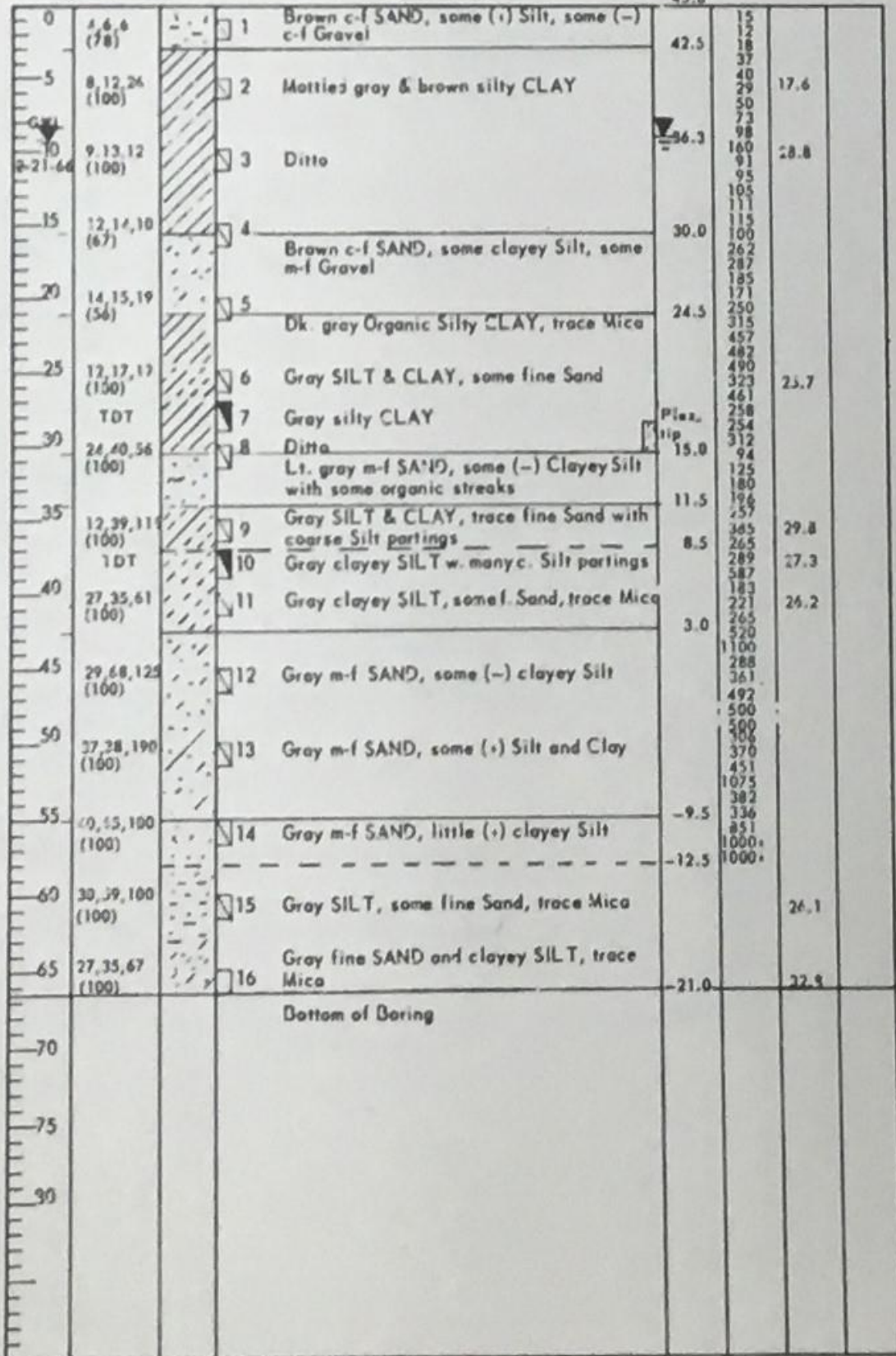
EL. 45.9

+45.9



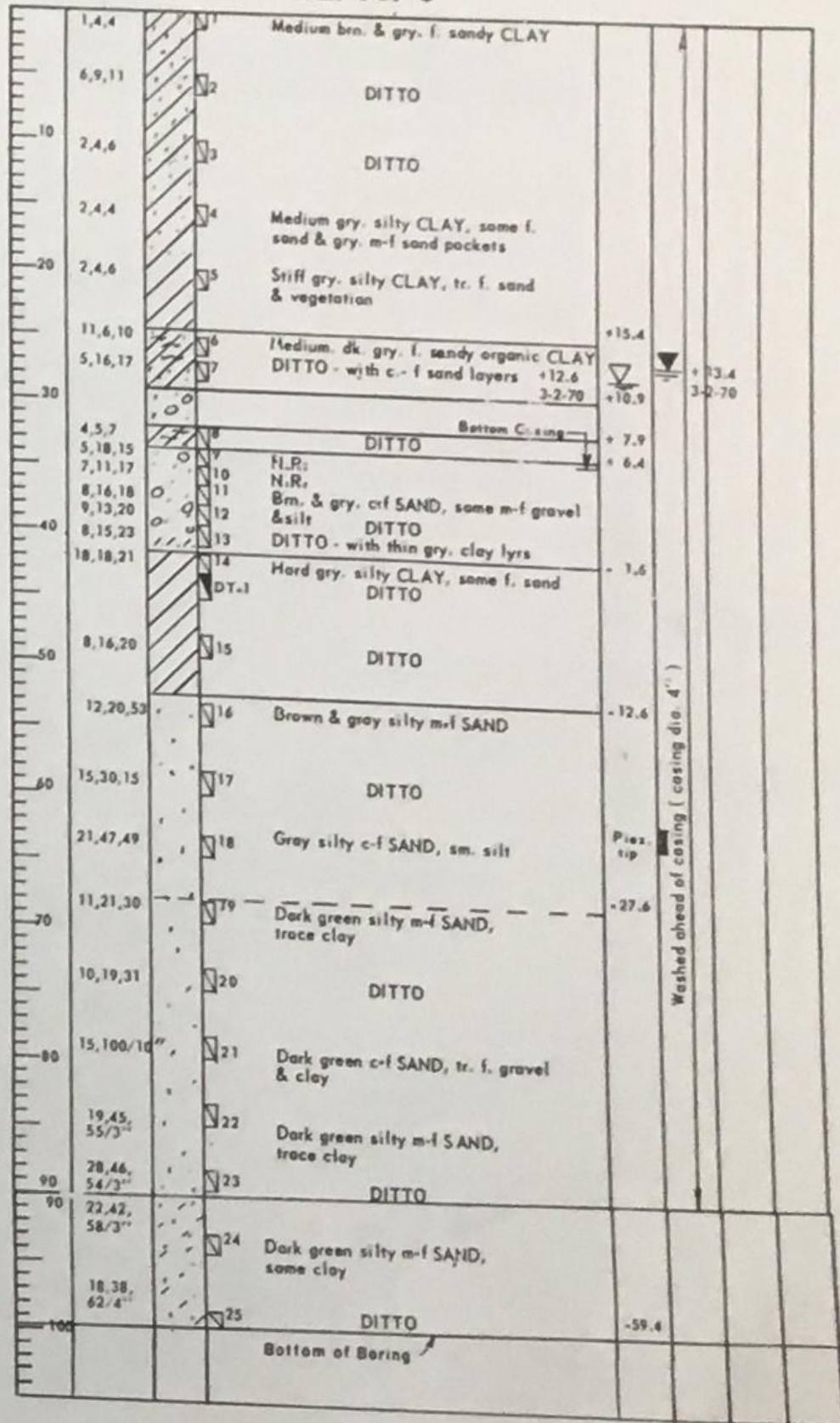
B-4-17

EL. 45.6

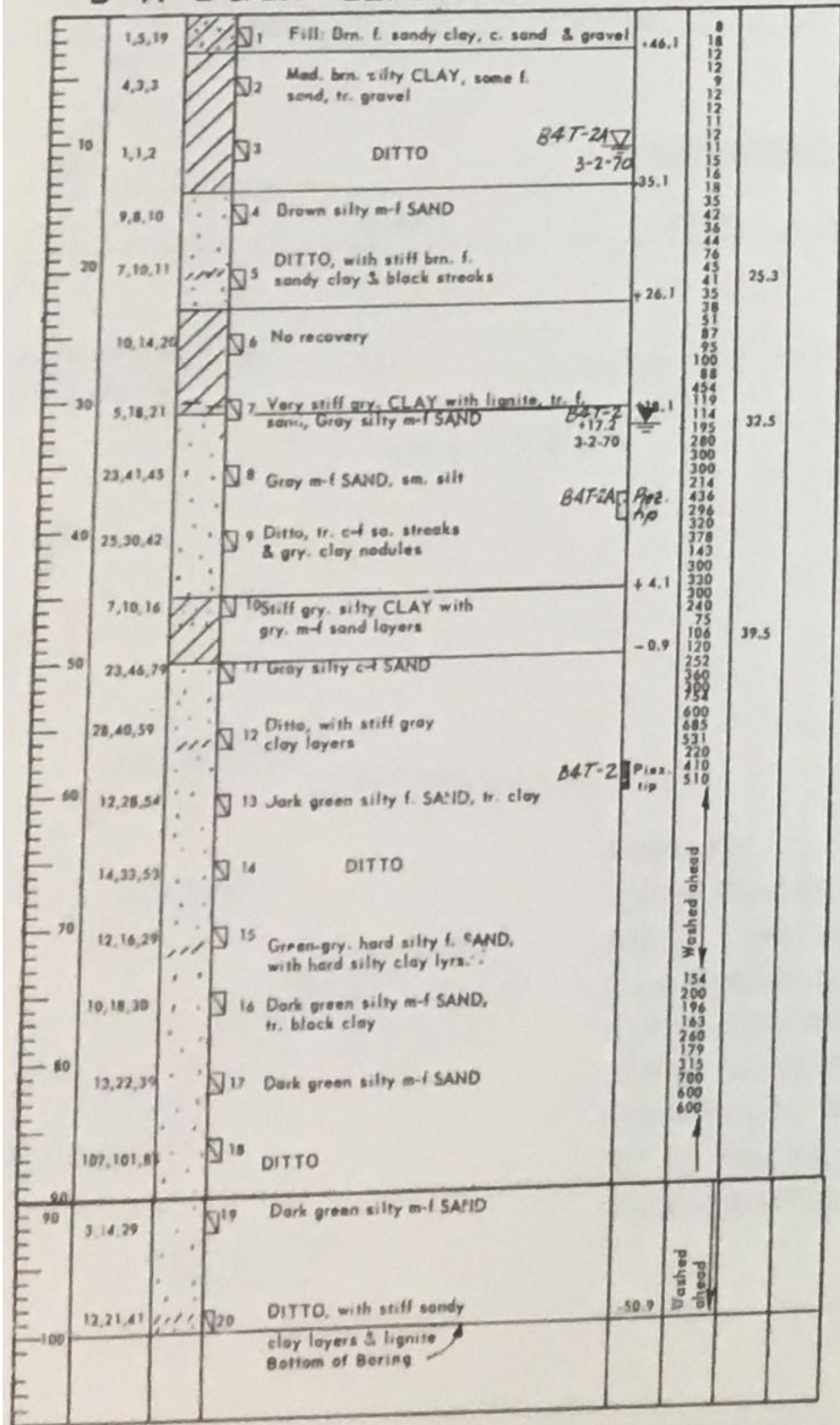


B-4T-1

EL. 39.9

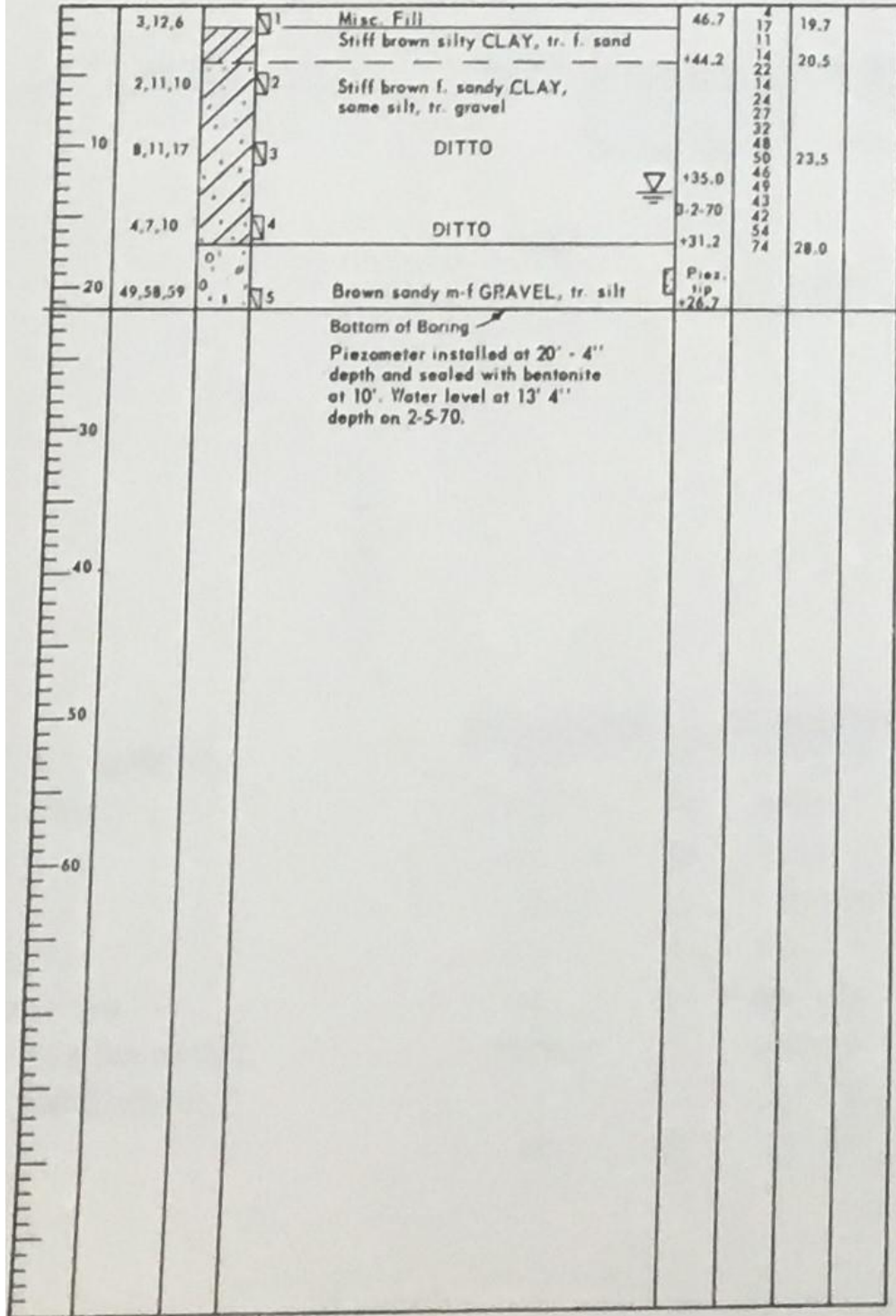


B-4T-2 & 2A EL. 49.1



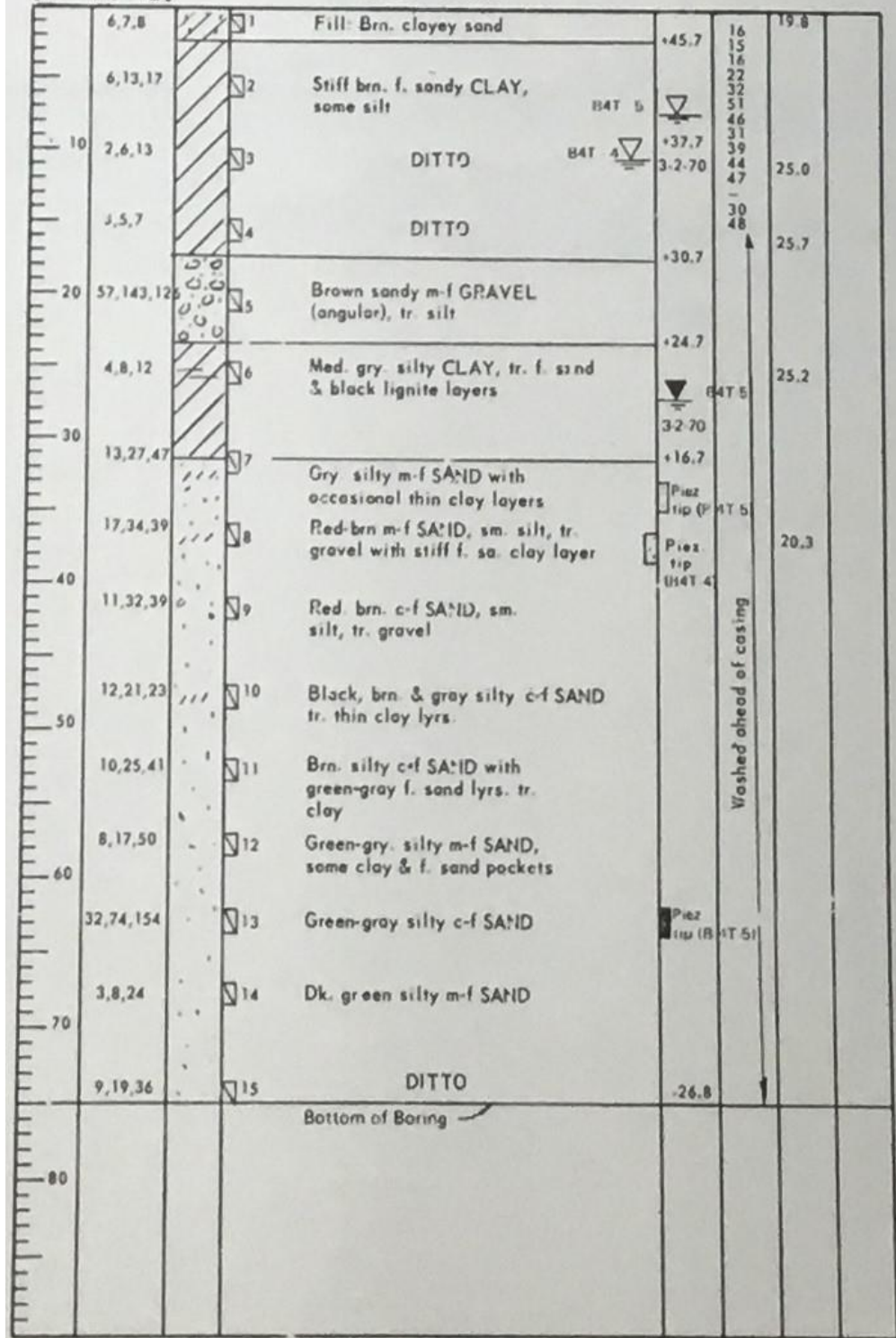
B-4T-3

EL. 48.2



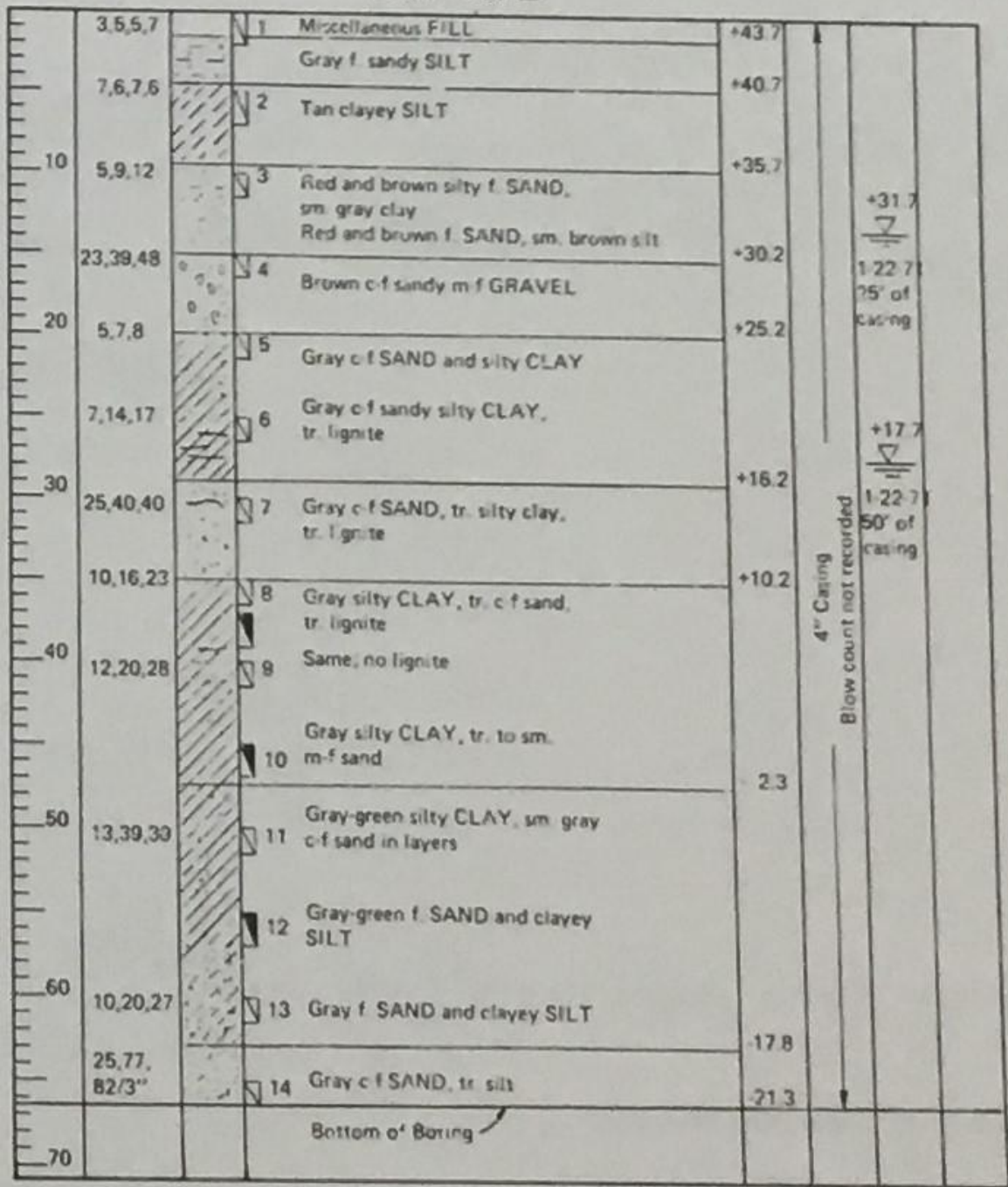
B-4T-4
(B-4T-5)

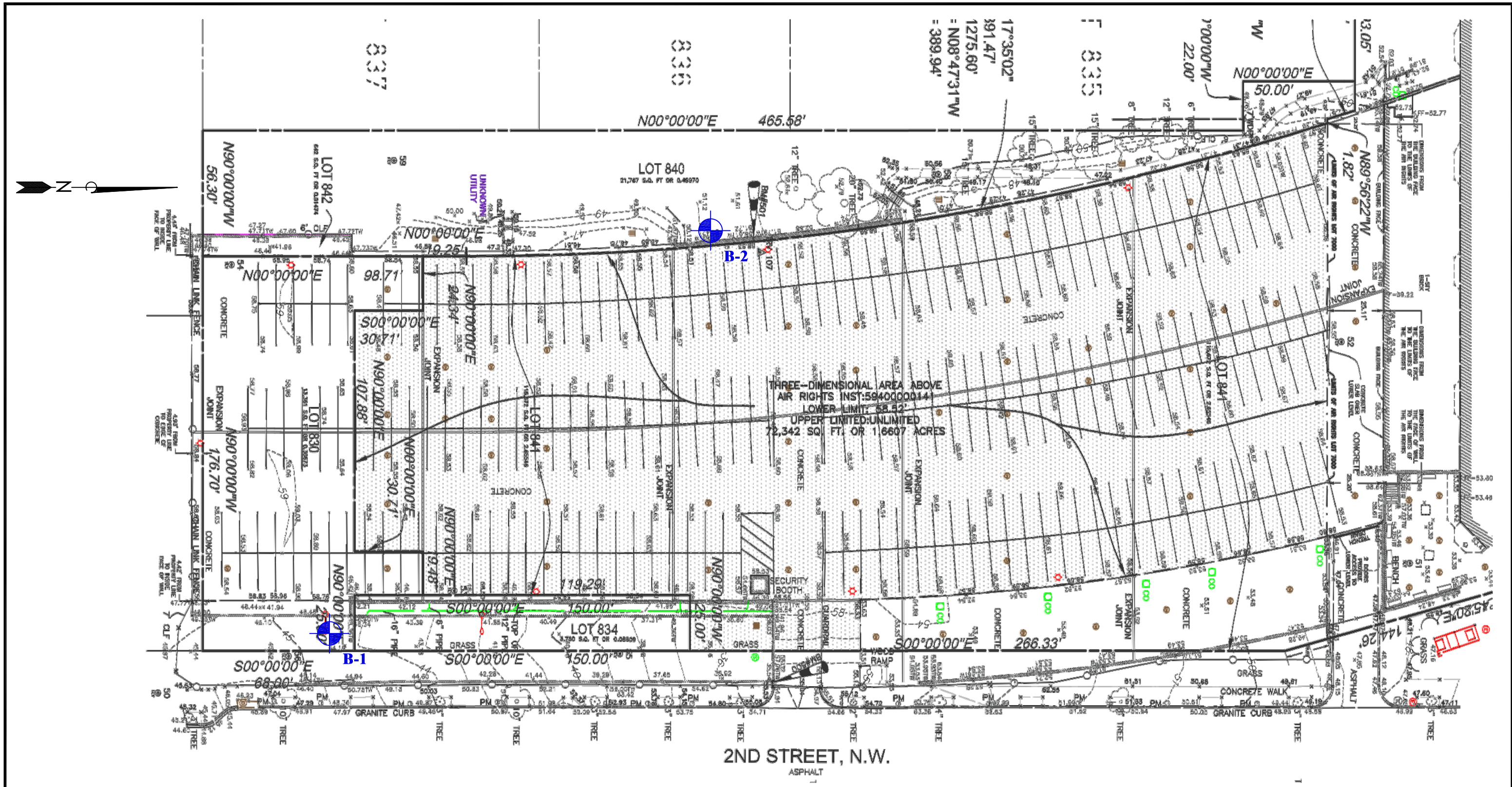
EL. 48.2



B-4T-6

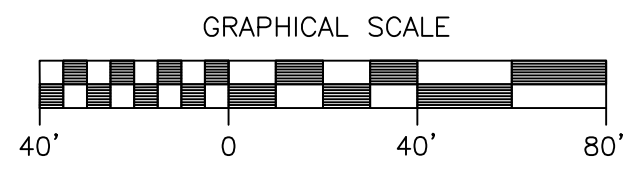
EL. 45.2






LEGEND

 APPROXIMATE BORING LOCATION
B-1



NOTE: BASE PLAN PROVIDED BY AMT LLC DATED AUGUST 2014.

G:\PROJECTS\Active DC16 Projects\DC16024, Parking Deck Analysis from ODMPED\CAD\BLP DC16024.dwg



**GeoCapitol
Engineering LLC**
 4545 42nd Street NW, Suite 307
 Washington, DC 20016 (202) 375-7900

200 K STREET NW PARKING DECK WASHINGTON, DC 20001		Scale: AS SHOWN	Fig. 2
BORING LOCATION PLAN			
Date: SEPT. 2016	Checked By: D.F.G.	Project No.: DC16024	